

**INTERSECTION CAPACITY UTILIZATION WORKSHEET**  
**COURTYARD & TOWNPLACE SUITES HOTEL PROJECT - #15068**  
COUNT DATE: 10/03/2013  
N/S STREET: KANAN ROAD  
E/W STREET: AGOURA ROAD  
TIME PERIOD: P.M. PEAK HOUR  
CONTROL TYPE: SIGNAL

REF. #10PM

**TRAFFIC VOLUME SUMMARY**

CONDITION	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	149	756	48	260	829	162	137	244	167	95	203	161
(B) PROJECT:	1	0	0	0	0	37	35	3	2	0	3	0
(C) NEAR TERM - ADDED:	15	7	9	58	12	63	59	42	14	11	35	57

GEOMETRICS: NORTH BOUND SOUTH BOUND EAST BOUND WEST BOUND  
L T TR L T R L TR L T R

MOVEMENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS			
			1	2	3	4	1	2	3	4
NBL	1	1600	149	150	164	165	0.093 *	0.094 *	0.103 *	0.103
NBT	2	3200	756	756	763	763	0.251	0.251	0.256	0.256
NBR	0	0	48	48	57	57	-	-	-	-
SBL	1	1600	260	260	318	318	0.163	0.163	0.199	0.199
SBT	1	1600	829	829	841	841	0.518 *	0.518 *	0.526 *	0.526 *
SBR	1	1600	162	199	225	262	0.101	0.124	0.141	0.164
EBL	1	1600	137	172	196	231	0.086	0.108	0.123	0.144
EBT	1	1600	244	247	286	289	0.257 *	0.260 *	0.292 *	0.295 *
EBR	0	0	167	169	181	183	-	-	-	-
WBL	1	1600	95	95	106	106	0.059 *	0.059 *	0.066 *	0.066 *
WBT	1	1600	203	206	238	241	0.127	0.129	0.149	0.151
WBR	1	1600	161	161	218	218	0.101	0.101	0.136	0.136
CLEARANCE INTERVAL:							0.05 *	0.05 *	0.05 *	0.05 *
INTERSECTION CAPACITY UTILIZATION:							0.98	0.98	1.04	1.04
LEVEL OF SERVICE:							E	E	F	F

- SCENARIO 1: EXISTING (A)
- SCENARIO 2: EXISTING+PROJECT (A+B)
- SCENARIO 3: EXISTING+NEAR-TERM (A+C)
- SCENARIO 4: EXISTING+PROJECT+NEAR-TERM (A+B+C)

NOTES:

INTERSECTION CAPACITY UTILIZATION WORKSHEET  
 COURTYARD & TOWNPLACE SUITES HOTEL PROJECT - #15068  
 COUNT DATE: 10/03/2013  
 N/S STREET: KANAN ROAD  
 E/W STREET: AGOURA ROAD  
 TIME PERIOD: A.M. PEAK HOUR  
 CONTROL TYPE: SIGNAL

REF. #10AM

**TRAFFIC VOLUME SUMMARY**

CONDITION	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	48	609	16	97	1050	251	89	70	96	50	62	59
(B) PROJECT:	1	0	0	0	0	37	26	2	1	0	4	0
(C) NEAR TERM - ADDED:	13	8	11	60	10	64	42	30	9	6	33	38

IMPROVED GEOMETRICS: NORTH BOUND SOUTH BOUND EAST BOUND WEST BOUND  
 L T TR LL TT R LL TR L T R

MOVEMENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS			
			1	2	3	4	1	2	3	4
NBL	1	1600	48	49	61	62	0.030	0.031	0.038	0.039
NBT	2	3200	609	609	617	617	0.195 *	0.195 *	0.201 *	0.201 *
NBR	0	0	16	16	27	27	-	-	-	-
SBL	2	3200	97	97	157	157	0.030	0.030	0.049	0.049
SBT	2	3200	1050	1050	1060	1060	0.328 *	0.328 *	0.331 *	0.331 *
SBR	1	1600	251	288	315	352	0.157	0.180	0.197	0.220
EBL	2	3200	89	115	131	157	0.028	0.036	0.041	0.049
EBT	1	1600	70	72	100	102	0.104 *	0.106 *	0.128 *	0.130 *
EBR	0	0	96	97	105	106	-	-	-	-
WBL	1	1600	50	50	56	56	0.031	0.031	0.035	0.035
WBT	1	1600	62	66	95	99	0.039 *	0.041 *	0.059 *	0.062 *
WBR	1	1600	59	59	97	97	0.037	0.037	0.061	0.061
CLEARANCE INTERVAL:							0.05 *	0.05 *	0.05 *	0.05 *
INTERSECTION CAPACITY UTILIZATION:							0.72	0.72	0.77	0.77
LEVEL OF SERVICE:							C	C	C	C

SCENARIO 1: EXISTING (A)  
 SCENARIO 2: EXISTING+PROJECT (A+B)  
 SCENARIO 3: EXISTING+NEAR-TERM (A+C)  
 SCENARIO 4: EXISTING+PROJECT+NEAR-TERM (A+B+C)

NOTES:

**INTERSECTION CAPACITY UTILIZATION WORKSHEET**  
**COURTYARD & TOWNPLACE SUITES HOTEL PROJECT - #15068**

REF. #10PM

COUNT DATE: 10/03/2013  
 N/S STREET: KANAN ROAD  
 E/W STREET: AGOURA ROAD  
 TIME PERIOD: P.M. PEAK HOUR  
 CONTROL TYPE: SIGNAL

**TRAFFIC VOLUME SUMMARY**

CONDITION	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) EXISTING:	149	756	48	260	829	162	137	244	167	95	203	161
(B) PROJECT:	1	0	0	0	0	37	35	3	2	0	3	0
(C) NEAR TERM - ADDED:	15	7	9	58	12	63	59	42	14	11	35	57

**IMPROVED GEOMETRICS:**

NORTH BOUND		SOUTH BOUND		EAST BOUND		WEST BOUND	
L	T TR	LL	TT R	LL	TR	L	T R

MOVEMENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS			
			1	2	3	4	1	2	3	4
NBL	1	1600	149	150	164	165	0.093 *	0.094 *	0.103 *	0.103 *
NBT	2	3200	756	756	763	763	0.251	0.251	0.256	0.256
NBR	0	0	48	48	57	57	-	-	-	-
SBL	2	3200	260	260	318	318	0.081	0.081	0.099	0.099
SBT	2	3200	829	829	841	841	0.259 *	0.259 *	0.263 *	0.263 *
SBR	1	1600	162	199	225	262	0.101	0.124	0.141	0.164
EBL	2	3200	137	172	196	231	0.043	0.054	0.061	0.072
EBT	1	1600	244	247	286	289	0.257 *	0.260 *	0.292 *	0.295 *
EBR	0	0	167	169	181	183	-	-	-	-
WBL	1	1600	95	95	106	106	0.059 *	0.059 *	0.066 *	0.066 *
WBT	1	1600	203	206	238	241	0.127	0.129	0.149	0.151
WBR	1	1600	161	161	218	218	0.101	0.101	0.136	0.136
<b>CLEARANCE INTERVAL:</b>							0.05 *	0.05 *	0.05 *	0.05 *
<b>INTERSECTION CAPACITY UTILIZATION:</b>							0.72	0.72	0.77	0.78
<b>LEVEL OF SERVICE:</b>							C	C	C	C

SCENARIO 1: EXISTING (A)  
 SCENARIO 2: EXISTING+PROJECT (A+B)  
 SCENARIO 3: EXISTING+NEAR-TERM (A+C)  
 SCENARIO 4: EXISTING+PROJECT+NEAR-TERM (A+B+C)

**NOTES:**

INTERSECTION CAPACITY UTILIZATION WORKSHEET

CORNERSTONE MIXED-USE PROJECT - #13070

REF. #10AM

COUNT DATE: 10/03/2013  
 N/S STREET: KANAN ROAD  
 E/W STREET: AGOURA ROAD  
 TIME PERIOD: A.M. PEAK HOUR  
 CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

CONDITION	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) CUMULATIVE:	69	710	29	172	1221	353	144	111	119	64	105	106
(B) PROJECT:	1	0	0	0	0	37	26	2	1	0	4	0

GEOMETRICS: NORTH BOUND SOUTH BOUND EAST BOUND WEST BOUND  
 L T T R LL T T R LL T R L T R

MOVEMENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO V/C RATIOS			
			1	2	3	4	1	2	3	4
NBL	1	1600	69	70	69	70	0.043 *	0.044 *	0.043 *	0.044 *
NBT	2	3200	710	710	710	710	0.222	0.222	0.222	0.222
NBR	1	1600	29	29	29	29	0.018	0.018	0.018	0.018
SBL	2	3200	172	172	172	172	0.054	0.054	0.054	0.054
SBT	2	3200	1221	1221	1221	1221	0.382 *	0.382 *	0.382 *	0.382 *
SBR	1	1600	353	390	353	390	0.221	0.244	0.221	0.244
EBL	2	3200	144	170	144	170	0.045	0.053	0.045	0.053
EBT	1	1600	111	113	111	113	0.069 *	0.071 *	0.069 *	0.071 *
EBR	1	1600	119	120	119	120	0.074	0.075	0.074	0.075
WBL	1	1600	64	64	64	64	0.040 *	0.040	0.040 *	0.040 *
WBT	1	1600	105	109	105	109	0.066	0.068 *	0.066	0.068
WBR	1	1600	106	106	106	106	0.066	0.066	0.066	0.066
CLEARANCE INTERVAL:							0.05 *	0.05 *	0.05 *	0.05 *
INTERSECTION CAPACITY UTILIZATION:							0.58	0.62	0.58	0.59
LEVEL OF SERVICE:							A	B	A	A

SCENARIO 1: CUMULATIVE (A)  
 SCENARIO 2: CUMULATIVE+PROJECT (A+B)

NOTES:



INTERSECTION CAPACITY UTILIZATION WORKSHEET

CORNERSTONE MIXED-USE PROJECT - #13070

REF. #10PM

COUNT DATE: 10/03/2013  
 N/S STREET: KANAN ROAD  
 EW STREET: AGOURA ROAD  
 TIME PERIOD: P.M. PEAK HOUR  
 CONTROL TYPE: SIGNAL

TRAFFIC VOLUME SUMMARY

CONDITION	NORTH BOUND			SOUTH BOUND			EAST BOUND			WEST BOUND		
	L	T	R	L	T	R	L	T	R	L	T	R
(A) CUMULATIVE:	187	879	65	358	969	250	217	323	207	120	270	243
(B) PROJECT:	1	0	0	0	0	37	35	3	2	0	3	0

GEOMETRICS:

NORTH BOUND SOUTH BOUND EAST BOUND WEST BOUND  
 L T T R LL T T R LL T R L T R

MOVEMENTS	# OF LANES	CAPACITY	SCENARIO VOLUMES				SCENARIO VC RATIOS			
			1	2	3	4	1	2	3	4
NBL	1	1600	187	188	187	188	0.117 *	0.118 *	0.117 *	0.118 *
NBT	2	3200	879	879	879	879	0.275	0.275	0.275	0.275
NBR	1	1600	65	65	65	65	0.041	0.041	0.041	0.041
SBL	2	3200	358	358	358	358	0.112	0.112	0.112	0.112
SBT	2	3200	969	969	969	969	0.303 *	0.303 *	0.303 *	0.303 *
SBR	1	1600	250	287	250	287	0.156	0.179	0.156	0.179
EBL	2	3200	217	252	217	252	0.068	0.079	0.068	0.079
EBT	1	1600	323	326	323	326	0.202 *	0.204 *	0.202 *	0.204 *
EBR	1	1600	207	209	207	209	0.129	0.131	0.129	0.131
WBL	1	1600	120	120	120	120	0.075 *	0.075 *	0.075 *	0.075 *
WBT	1	1600	270	273	270	273	0.169	0.171	0.169	0.171
WBR	1	1600	243	243	243	243	0.152	0.152	0.152	0.152

CLEARANCE INTERVAL: 0.05 \* 0.05 \* 0.05 \* 0.05 \*

INTERSECTION CAPACITY UTILIZATION: 0.75 0.75 0.75 0.75  
 LEVEL OF SERVICE: C C C C

SCENARIO 1: CUMULATIVE (A)  
 SCENARIO 2: CUMULATIVE+PROJECT (A+B)

NOTES:

## ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	Darryl F. Nelson	Intersection	Agoura Rd./Cirnell Rd.
Agency/Co.	ATE	Jurisdiction	City of Agoura Hills
Date Performed	8/27/2015	Analysis Year	Existing Conditions
Analysis Time Period	A.M. Peak Hour		

Project ID *Courtyard & Townplace Suites Hotel Project - #15068*

East/West Street: *Agoura Road*

North/South Street: *Cornell Road*

### Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	10	163	4	17	116	12
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	0	4	37	10	5	11
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LTR		LTR		LTR		LTR	
PHF	1.00		1.00		1.00		1.00	
Flow Rate (veh/h)	177		145		41		26	
% Heavy Vehicles	2		2		2		2	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

### Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.1		0.1		0.0		0.4	
Prop. Right-Turns	0.0		0.1		0.9		0.4	
Prop. Heavy Vehicle	0.0		0.0		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.0		0.0		-0.5		-0.1	

### Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.16		0.13		0.04		0.02	
hd, final value (s)	4.23		4.24		4.13		4.51	
x, final value	0.21		0.17		0.05		0.03	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t <sub>s</sub> (s)	2.2		2.2		2.1		2.5	

### Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	427		395		291		276	
Delay (s/veh)	8.34		8.11		7.34		7.67	
LOS	A		A		A		A	
Approach: Delay (s/veh)	8.34		8.11		7.34		7.67	
LOS	A		A		A		A	
Intersection Delay (s/veh)	8.10							
Intersection LOS	A							



## ALL-WAY STOP CONTROL ANALYSIS

### General Information

Analyst	Darryl F. Nelson
Agency/Co.	ATE
Date Performed	8/27/2015
Analysis Time Period	P.M. Peak Hour

### Site Information

Intersection	Agoura Rd./Cornell Rd.
Jurisdiction	City of Agoura Hills
Analysis Year	Existing Conditions

Project ID *Courtyard & Townplace Suites Hotel Project - #15068*

East/West Street: *Agoura Road*

North/South Street: *Cornell Road*

### Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	37	495	17	68	361	23
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	18	20	43	33	20	52
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LTR		LTR		LTR		LTR	
PHF	1.00		1.00		1.00		1.00	
Flow Rate (veh/h)	549		452		81		105	
% Heavy Vehicles	2		2		2		2	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

### Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.1		0.2		0.2		0.3	
Prop. Right-Turns	0.0		0.1		0.5		0.5	
Prop. Heavy Vehicle	0.0		0.0		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.0		0.0		-0.2		-0.2	

### Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.49		0.40		0.07		0.09	
hd, final value (s)	5.24		5.37		6.53		6.49	
x, final value	0.80		0.67		0.15		0.19	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t <sub>s</sub> (s)	3.2		3.4		4.5		4.5	

### Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	677		653		331		355	
Delay (s/veh)	25.67		18.73		10.65		11.00	
LOS	D		C		B		B	
Approach: Delay (s/veh)	25.67		18.73		10.65		11.00	
LOS	D		C		B		B	
Intersection Delay (s/veh)	20.70							
Intersection LOS	C							

## ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	Darryl F. Nelson	Intersection	Agoura Rd./Cirnell Rd.
Agency/Co.	ATE	Jurisdiction	City of Agoura Hills
Date Performed	8/27/2015	Analysis Year	Existing + Project
Analysis Time Period	A.M. Peak Hour		

Project ID Courtyard & Townplace Suites Hotel Project - #15068

East/West Street: Agoura Road

North/South Street: Cornell Road

### Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	10	165	4	17	120	12
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	0	4	37	10	5	11
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LTR		LTR		LTR		LTR	
PHF	1.00		1.00		1.00		1.00	
Flow Rate (veh/h)	179		149		41		26	
% Heavy Vehicles	2		2		2		2	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

### Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.1		0.1		0.0		0.4	
Prop. Right-Turns	0.0		0.1		0.9		0.4	
Prop. Heavy Vehicle	0.0		0.0		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.0		0.0		-0.5		-0.1	

### Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.16		0.13		0.04		0.02	
hd, final value (s)	4.24		4.24		4.15		4.53	
x, final value	0.21		0.18		0.05		0.03	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t <sub>g</sub> (s)	2.2		2.2		2.1		2.5	

### Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	429		399		291		276	
Delay (s/veh)	8.36		8.15		7.35		7.68	
LOS	A		A		A		A	
Approach: Delay (s/veh)	8.36		8.15		7.35		7.68	
LOS	A		A		A		A	
Intersection Delay (s/veh)	8.13							
Intersection LOS	A							



## ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	Darryl F. Nelson	Intersection	Agoura Rd./Cimell Rd.
Agency/Co.	ATE	Jurisdiction	City of Agoura Hills
Date Performed	8/27/2015	Analysis Year	Existing + Project
Analysis Time Period	P.M. Peak Hour		

Project ID Courtyard & Townplace Suites Hotel Project - #15068

East/West Street: Agoura Road

North/South Street: Cornell Road

### Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	37	498	17	68	364	23
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	18	20	43	33	20	52
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LTR		LTR		LTR		LTR	
PHF	1.00		1.00		1.00		1.00	
Flow Rate (veh/h)	552		455		81		105	
% Heavy Vehicles	2		2		2		2	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

### Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.1		0.1		0.2		0.3	
Prop. Right-Turns	0.0		0.1		0.5		0.5	
Prop. Heavy Vehicle	0.0		0.0		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.0		0.0		-0.2		-0.2	

### Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.49		0.40		0.07		0.09	
hd, final value (s)	5.25		5.38		6.55		6.51	
x, final value	0.80		0.68		0.15		0.19	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t <sub>s</sub> (s)	3.2		3.4		4.6		4.5	

### Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	677		653		331		355	
Delay (s/veh)	26.15		18.99		10.68		11.02	
LOS	D		C		B		B	
Approach: Delay (s/veh)	26.15		18.99		10.68		11.02	
LOS	D		C		B		B	
Intersection Delay (s/veh)	21.04							
Intersection LOS	C							

## ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	Darryl F. Nelson	Intersection	Agoura Rd./Cornell Rd.
Agency/Co.	ATE	Jurisdiction	City of Agoura Hills
Date Performed	8/27/2015	Analysis Year	Near-Term Conditions
Analysis Time Period	A.M. Peak Hour		

Project ID *Courtyard & Townplace Suites Hotel Project - #15068*

East/West Street: *Agoura Road*

North/South Street: *Cornell Road*

### Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	14	255	11	24	186	23
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	8	9	41	30	7	13
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	<i>LTR</i>		<i>LTR</i>		<i>LTR</i>		<i>LTR</i>	
PHF	1.00		1.00		1.00		1.00	
Flow Rate (veh/h)	280		233		58		50	
% Heavy Vehicles	2		2		2		2	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

### Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.1		0.1		0.1		0.6	
Prop. Right-Turns	0.0		0.1		0.7		0.3	
Prop. Heavy Vehicle	0.0		0.0		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.0		-0.0		-0.4		-0.0	

### Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.25		0.21		0.05		0.04	
hd, final value (s)	4.46		4.49		4.79		5.16	
x, final value	0.35		0.29		0.08		0.07	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t <sub>s</sub> (s)	2.5		2.5		2.8		3.2	

### Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	530		483		308		300	
Delay (s/veh)	9.82		9.32		8.19		8.56	
LOS	A		A		A		A	
Approach: Delay (s/veh)	9.82		9.32		8.19		8.56	
LOS	A		A		A		A	
Intersection Delay (s/veh)	9.38							
Intersection LOS	A							



## ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	Darryl F. Nelson	Intersection	Agoura Rd./Cornell Rd.
Agency/Co.	ATE	Jurisdiction	City of Agoura Hills
Date Performed	8/27/2015	Analysis Year	Near-Term Conditions
Analysis Time Period	P.M. Peak Hour		

Project ID *Courtyard & Townplace Suites Hotel Project - #15068*

East/West Street: *Agoura Road*

North/South Street: *Cornell Road*

### Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	42	598	22	73	453	38
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	30	35	49	53	23	53
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	<i>LTR</i>		<i>LTR</i>		<i>LTR</i>		<i>LTR</i>	
PHF	1.00		1.00		1.00		1.00	
Flow Rate (veh/h)	662		564		114		129	
% Heavy Vehicles	2		2		2		2	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

### Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.1		0.1		0.3		0.4	
Prop. Right-Turns	0.0		0.1		0.4		0.4	
Prop. Heavy Vehicle	0.0		0.0		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.0		0.0		-0.2		-0.1	

### Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.59		0.50		0.10		0.11	
hd, final value (s)	5.92		5.98		7.56		7.53	
x, final value	1.09		0.94		0.24		0.27	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t <sub>s</sub> (s)	3.9		4.0		5.6		5.5	

### Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	662		600		364		379	
Delay (s/veh)	86.16		47.02		12.93		13.28	
LOS	F		E		B		B	
Approach: Delay (s/veh)	86.16		47.02		12.93		13.28	
LOS	F		E		B		B	
Intersection Delay (s/veh)	59.05							
Intersection LOS	F							



## ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	Darryl F. Nelson	Intersection	Agoura Rd./Cornell Rd.
Agency/Co.	ATE	Jurisdiction	City of Agoura Hills
Date Performed	8/27/2015	Analysis Year	Near-Term + Project
Analysis Time Period	A.M. Peak Hour		

Project ID *Courtyard & Townplace Suites Hotel Project - #15068*

East/West Street: *Agoura Road*      North/South Street: *Cornell Road*

### Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	14	257	11	24	190	23
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	8	9	41	30	7	13
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	<i>LTR</i>		<i>LTR</i>		<i>LTR</i>		<i>LTR</i>	
PHF	1.00		1.00		1.00		1.00	
Flow Rate (veh/h)	282		237		58		50	
% Heavy Vehicles	2		2		2		2	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

### Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.0	0.1	0.1	0.6
Prop. Right-Turns	0.0	0.1	0.7	0.3
Prop. Heavy Vehicle	0.0	0.0	0.0	0.0
hLT-adj	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7
hadj, computed	0.0	-0.0	-0.4	-0.0

### Departure Headway and Service Time

hd, initial value (s)	3.20	3.20	3.20	3.20
x, initial	0.25	0.21	0.05	0.04
hd, final value (s)	4.47	4.49	4.80	5.17
x, final value	0.35	0.30	0.08	0.07
Move-up time, m (s)	2.0		2.0	
Service Time, t <sub>s</sub> (s)	2.5	2.5	2.8	3.2

### Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	532		487		308		300	
Delay (s/veh)	9.86		9.37		8.21		8.57	
LOS	A		A		A		A	
Approach: Delay (s/veh)	9.86		9.37		8.21		8.57	
LOS	A		A		A		A	
Intersection Delay (s/veh)	9.42							
Intersection LOS	A							

## ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	Darryl F. Nelson	Intersection	Agoura Rd./Cornell Rd.
Agency/Co.	ATE	Jurisdiction	City of Agoura Hills
Date Performed	8/27/2015	Analysis Year	Near-Term + Project
Analysis Time Period	P.M. Peak Hour		

Project ID *Courtyard & Townplace Suites Hotel Project - #15068*

East/West Street: *Agoura Road*

North/South Street: *Cornell Road*

### Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	42	601	22	73	456	38
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	30	35	49	53	23	53
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	<i>LTR</i>		<i>LTR</i>		<i>LTR</i>		<i>LTR</i>	
PHF	1.00		1.00		1.00		1.00	
Flow Rate (veh/h)	665		567		114		129	
% Heavy Vehicles	2		2		2		2	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

### Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.1		0.1		0.3		0.4	
Prop. Right-Turns	0.0		0.1		0.4		0.4	
Prop. Heavy Vehicle	0.0		0.0		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.0		0.0		-0.2		-0.1	

### Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.59		0.50		0.10		0.11	
hd, final value (s)	5.93		5.98		7.57		7.54	
x, final value	1.09		0.94		0.24		0.27	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t <sub>s</sub> (s)	3.9		4.0		5.6		5.5	

### Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	665		600		364		379	
Delay (s/veh)	88.27		48.04		12.94		13.30	
LOS	<i>F</i>		<i>E</i>		<i>B</i>		<i>B</i>	
Approach: Delay (s/veh)	88.27		48.04		12.94		13.30	
LOS	<i>F</i>		<i>E</i>		<i>B</i>		<i>B</i>	
Intersection Delay (s/veh)	60.43							
Intersection LOS	<i>F</i>							



## ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	Darryl F. Nelson	Intersection	Agoura Rd./Cimell Rd.
Agency/Co.	ATE	Jurisdiction	City of Agoura Hills
Date Performed	8/27/2015	Analysis Year	Cumulative Conditions
Analysis Time Period	A.M. Peak Hour		

Project ID *Courtyard & Townplace Suites Hotel Project - #15068*

East/West Street: *Agoura Road*

North/South Street: *Cornell Road*

### Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	17	280	12	27	204	25
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	13	10	47	32	8	15
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LTR		LTR		LTR		LTR	
PHF	1.00		1.00		1.00		1.00	
Flow Rate (veh/h)	309		256		70		55	
% Heavy Vehicles	2		2		2		2	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

### Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.1		0.1		0.2		0.6	
Prop. Right-Turns	0.0		0.1		0.7		0.3	
Prop. Heavy Vehicle	0.0		0.0		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.0		-0.0		-0.3		-0.0	

### Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.27		0.23		0.06		0.05	
hd, final value (s)	4.56		4.59		4.97		5.32	
x, final value	0.39		0.33		0.10		0.08	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t <sub>s</sub> (s)	2.6		2.6		3.0		3.3	

### Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	559		506		320		305	
Delay (s/veh)	10.46		9.80		8.51		8.79	
LOS	B		A		A		A	
Approach: Delay (s/veh)	10.46		9.80		8.51		8.79	
LOS	B		A		A		A	
Intersection Delay (s/veh)	9.88							
Intersection LOS	A							



## ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	Darryl F. Nelson	Intersection	Agoura Rd./Cornell Rd.
Agency/Co.	ATE	Jurisdiction	City of Agoura Hills
Date Performed	8/27/2015	Analysis Year	Cumulative Conditions
Analysis Time Period	P.M. Peak Hour		

Project ID *Courtyard & Townplace Suites Hotel Project - #15068*

East/West Street: *Agoura Road*

North/South Street: *Cornell Road*

### Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	48	674	25	84	508	42
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	33	38	56	58	26	61
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	<i>LTR</i>		<i>LTR</i>		<i>LTR</i>		<i>LTR</i>	
PHF	1.00		1.00		1.00		1.00	
Flow Rate (veh/h)	747		634		127		145	
% Heavy Vehicles	2		2		2		2	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

### Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.1		0.1		0.3		0.4	
Prop. Right-Turns	0.0		0.1		0.4		0.4	
Prop. Heavy Vehicle	0.0		0.0		0.0		0.0	
hLT-adj	0.2	-0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.0		0.0		-0.2		-0.1	

### Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.66		0.56		0.11		0.13	
hd, final value (s)	6.18		6.17		7.77		7.72	
x, final value	1.28		1.09		0.27		0.31	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t <sub>s</sub> (s)	4.2		4.2		5.8		5.7	

### Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	747		634		377		395	
Delay (s/veh)	159.69		87.03		13.68		14.16	
LOS	<i>F</i>		<i>F</i>		<i>B</i>		<i>B</i>	
Approach: Delay (s/veh)	159.69		87.03		13.68		14.16	
LOS	<i>F</i>		<i>F</i>		<i>B</i>		<i>B</i>	
Intersection Delay (s/veh)	107.84							
Intersection LOS	<i>F</i>							

## ALL-WAY STOP CONTROL ANALYSIS

### General Information

Analyst	Darryl F. Nelson
Agency/Co.	ATE
Date Performed	8/27/2015
Analysis Time Period	A.M. Peak Hour

### Site Information

Intersection	Agoura Rd./Cimell Rd.
Jurisdiction	City of Agoura Hills
Analysis Year	Cumulative + Project

Project ID *Courtyard & Townplace Suites Hotel Project - #15068*

East/West Street: *Agoura Road*

North/South Street: *Cornell Road*

### Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	17	282	12	27	208	25
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	13	10	47	32	8	15
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LTR		LTR		LTR		LTR	
PHF	1.00		1.00		1.00		1.00	
Flow Rate (veh/h)	311		260		70		55	
% Heavy Vehicles	2		2		2		2	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

### Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.1		0.1		0.2		0.6	
Prop. Right-Turns	0.0		0.1		0.7		0.3	
Prop. Heavy Vehicle	0.0		0.0		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.0		-0.0		-0.3		-0.0	

### Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.28		0.23		0.06		0.05	
hd, final value (s)	4.57		4.60		4.99		5.33	
x, final value	0.39		0.33		0.10		0.08	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t <sub>g</sub> (s)	2.6		2.6		3.0		3.3	

### Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	561		510		320		305	
Delay (s/veh)	10.51		9.87		8.52		8.80	
LOS	B		A		A		A	
Approach: Delay (s/veh)	10.51		9.87		8.52		8.80	
LOS	B		A		A		A	
Intersection Delay (s/veh)	9.93							
Intersection LOS	A							



## ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	Darryl F. Nelson	Intersection	Agoura Rd./Cimell Rd.
Agency/Co.	ATE	Jurisdiction	City of Agoura Hills
Date Performed	8/27/2015	Analysis Year	Cumulative + Project
Analysis Time Period	P.M. Peak Hour		

Project ID Courtyard & Townplace Suites Hotel Project - #15068

East/West Street: Agoura Road

North/South Street: Cornell Road

### Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	48	677	25	84	511	42
%Thrus Left Lane						

Approach	Northbound			Southbound		
	L	T	R	L	T	R
Movement						
Volume (veh/h)	33	38	56	58	26	61
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LTR		LTR		LTR		LTR	
PHF	1.00		1.00		1.00		1.00	
Flow Rate (veh/h)	750		637		127		145	
% Heavy Vehicles	2		2		2		2	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

### Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.1		0.1		0.3		0.4	
Prop. Right-Turns	0.0		0.1		0.4		0.4	
Prop. Heavy Vehicle	0.0		0.0		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	0.0		0.0		-0.2		-0.1	

### Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.67		0.57		0.11		0.13	
hd, final value (s)	6.18		6.17		7.77		7.72	
x, final value	1.29		1.09		0.27		0.31	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t <sub>g</sub> (s)	4.2		4.2		5.8		5.7	

### Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	750		637		377		395	
Delay (s/veh)	161.78		88.71		13.68		14.16	
LOS	F		F		B		B	
Approach: Delay (s/veh)	161.78		88.71		13.68		14.16	
LOS	F		F		B		B	
Intersection Delay (s/veh)	109.49							
Intersection LOS	F							



## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	Darryl F. Nelson	Intersection	Agoura Rd./Project Dwy.
Agency/Co.	ATE	Jurisdiction	City of Agoura Hills
Date Performed	8/27/2015	Analysis Year	Cumulative + Project
Analysis Time Period	A.M. Peak Hour		

Project Description *Courtyard & Townplace Suites Hotel Project - #15068*

East/West Street: *Agoura Road*

North/South Street: *Project Driveway*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		290			478	42
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	0	290	0	0	478	42
Percent Heavy Vehicles	2	--	--	0	--	--
Median Type	<i>Raised curb</i>					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration		T			T	TR
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)						20
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	20
Percent Heavy Vehicles	0	0	0	2	0	2
Percent Grade (%)		0			0	
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	1
Configuration						R

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration								R
v (veh/h)								20
C (m) (veh/h)								777
v/c								0.03
95% queue length								0.08
Control Delay (s/veh)								9.8
LOS								A
Approach Delay (s/veh)	---	---					9.8	
Approach LOS	---	---					A	

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	Darryl F. Nelson	Intersection	Agoura Rd./Project Dwy.
Agency/Co.	ATE	Jurisdiction	City of Agoura Hills
Date Performed	8/27/2015	Analysis Year	Cumulative + Project
Analysis Time Period	P.M. Peak Hour		

Project Description *Courtyard & Townplace Suites Hotel Project - #15068*

East/West Street: *Agoura Road*

North/South Street: *Project Driveway*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound			
	Movement	1	2	3	4	5	6
		L	T	R	L	T	R
Volume (veh/h)			612			455	41
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	0	612	0	0	455	41	
Percent Heavy Vehicles	2	--	--	0	--	--	
Median Type	<i>Raised curb</i>						
RT Channelized			0			0	
Lanes	0	2	0	0	2	0	
Configuration		T			T	TR	
Upstream Signal		0			0		

Minor Street	Northbound			Southbound			
	Movement	7	8	9	10	11	12
		L	T	R	L	T	R
Volume (veh/h)							26
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0	26
Percent Heavy Vehicles	0	0	0	2	0	0	2
Percent Grade (%)		0			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0				0
Lanes	0	0	0	0	0	0	1
Configuration							R

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
			7	8	9	10	11	12
Movement	1	4						
Lane Configuration								R
v (veh/h)								26
C (m) (veh/h)								789
v/c								0.03
95% queue length								0.10
Control Delay (s/veh)								9.7
LOS								A
Approach Delay (s/veh)	--	--						9.7
Approach LOS	--	--						A



## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	Darryl F. Nelson	Intersection	Roadside Rd./Driveway
Agency/Co.	ATE	Jurisdiction	City of Agoura Hills
Date Performed	8/27/2015	Analysis Year	Cumulative + Project
Analysis Time Period	A.M. Peak Hour		

Project Description <i>Courtyard &amp; Townplace Suites Hotel Project - #15068</i>	
East/West Street: <i>Driveway</i>	North/South Street: <i>Roadside Road</i>
Intersection Orientation: <i>North-South</i>	Study Period (hrs): <i>0.25</i>

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	101	32			28	0
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	101	32	0	0	28	0
Percent Heavy Vehicles	0	--	--	2	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	LT					TR
Upstream Signal		0			0	

Minor Street	Eastbound			Westbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)	0	0	91			
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	0	0	91	0	0	0
Percent Heavy Vehicles	2	0	0	0	0	0
Percent Grade (%)		0			0	
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	0	0	0	0
Configuration		LTR				

Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LTR	
v (veh/h)	101						91	
C (m) (veh/h)	1599						1053	
v/c	0.06						0.09	
95% queue length	0.20						0.28	
Control Delay (s/veh)	7.4						8.7	
LOS	A						A	
Approach Delay (s/veh)	--	--					8.7	
Approach LOS	--	--					A	

Average weighted Delay = 8.0 sec.

LOS B

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	Darryl F. Nelson	Intersection	Roadside Rd./Driveway
Agency/Co.	ATE	Jurisdiction	City of Agoura Hills
Date Performed	8/27/2015	Analysis Year	Cumulative + Project
Analysis Time Period	P.M. Peak Hour		

Project Description: *Courtyard & Townplace Suites Hotel Project - #15068*  
 East/West Street: *Driveway*      North/South Street: *Roadside Road*  
 Intersection Orientation: *North-South*      Study Period (hrs): *0.25*

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	114	27			63	0
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	114	27	0	0	63	0
Percent Heavy Vehicles	0	--	--	2	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	<i>LT</i>					<i>TR</i>
Upstream Signal		0			0	

Minor Street	Eastbound			Westbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)	0	0	118			
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	0	0	118	0	0	0
Percent Heavy Vehicles	2	0	0	0	0	0
Percent Grade (%)		0			0	
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	0	0	0	0
Configuration		<i>LTR</i>				

Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>						<i>LTR</i>	
v (veh/h)	114						118	
C (m) (veh/h)	1553						1007	
v/c	0.07						0.12	
95% queue length	0.24						0.40	
Control Delay (s/veh)	7.5						9.0	
LOS	<i>A</i>						<i>A</i>	
Approach Delay (s/veh)	--	--					9.0	
Approach LOS	--	--					<i>A</i>	

Average Weighted Delay = 8.2 sec

LOS A



APPROVED/PENDING PROJECTS TRIP GENERATION SPREADSHEET

CUMULATIVE PROJECT TRIP GENERATION (#15068)

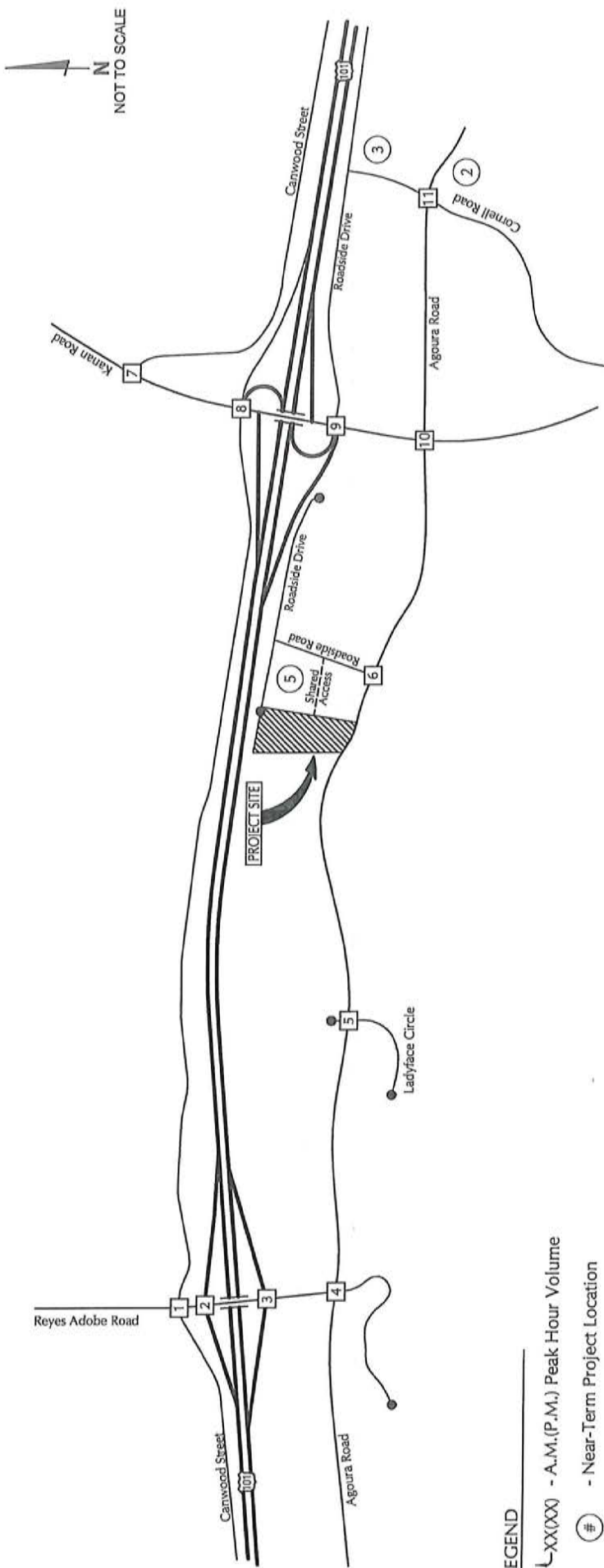
Project Name	Land-Use (ITE Code #)	Size	Multi-Trip	ADT		A.M. Peak Hour			P.M. Peak Hour						
				Rate	Trips	In %	Trips	Out %	Trips	In %	Trips	Out %	Trips		
1 Heathcote	Medical Office (#720)	14,075	1.00	36.13	509	79%	27	21%	7	3.57	50	28%	14	72%	36
2 Cornerstone Mixed-Use	N/A	N/A	1.00	N/A	3,035	0%	135	0%	85	N/A	242	0%	114	0%	128
3 Whizin Market	N/A	N/A	1.00	N/A	4,274	0%	137	0%	97	N/A	319	0%	170	0%	149
4 Utopia Hills	Restaurant (#932)	3,381	1.00	127.15	430	55%	20	45%	16	9.85	33	60%	20	40%	13
	Townhome (#230)	8	1.00	5.81	46	17%	1	83%	3	0.52	4	67%	3	33%	1
	Condominium (#230)	9	1.00	5.81	52	17%	1	83%	3	0.52	5	67%	3	33%	2
5 Selleck Development	Gym/Yoga (#939)	2,204	1.00	32.93	72	1.41	2	50%	1	3.53	8	57%	5	43%	3
6 Agoura Landmark	NA	NA	1.00	N/A	3,630	0%	111	0%	84	N/A	237	0%	131	0%	106
	Warehouse (#150)	48,532	1.00	3.56	173	0%	11	21%	3	0.32	16	25%	4	75%	12
	Office (#710)	21,320	1.00	11.03	235	88%	29	12%	4	1.49	32	17%	5	83%	27
7 Shirvanian	Industrial Park (#110)	103,000	1.00	6.97	718	88%	84	12%	11	0.97	100	12%	12	88%	88
8 Agoura Business Center	Commercial (#826)	21,782	1.00	44.32	965	50%	15	50%	14	2.71	59	44%	26	56%	33
9 APB	Office (#710)	30,400	1.00	11.03	335	88%	41	12%	6	1.49	45	17%	8	83%	37
10 Khanizis/Rice	Residential (#230)	46	1.00	5.81	267	0.44	20	17%	3	0.52	24	67%	16	33%	8
11 Jay Rodgers	Residential (#210)	18	1.00	9.52	171	14	4	75%	10	1.00	18	63%	11	37%	7
12 Barry Robles	Residential (#210)	2	1.00	9.52	19	0.75	2	25%	1	1.00	2	63%	1	37%	1
13 Payan	Residential (#210)	1	1.00	9.52	10	0.75	1	25%	1	1.00	1	63%	1	37%	0
14 Nabillah Moallem	Residential (#210)	1	1.00	9.52	10	0.75	1	25%	1	1.00	1	63%	1	37%	0
15 Katherine Neff	Residential (#210)	1	1.00	9.52	10	0.75	1	25%	1	1.00	1	63%	1	37%	0
16 Abudalu	Residential (#210)	1	1.00	9.52	10	0.75	1	25%	1	1.00	1	63%	1	37%	0
17 Texidor	Residential (#210)	1	1.00	9.52	10	0.75	1	25%	1	1.00	1	63%	1	37%	0
18 Gold	Residential (#210)	1	1.00	9.52	10	0.74	1	25%	1	1.00	1	63%	1	37%	0
<b>Total:</b>				<b>14,991</b>	<b>990</b>	<b>622</b>	<b>368</b>	<b>1,200</b>	<b>549</b>	<b>651</b>					



APPROVED/PENDING PROJECTS TRIP DISTRIBUTION







**LEGEND**

↳ XXXXX - A.M.(P.M.) Peak Hour Volume

Ⓝ - Near-Term Project Location

1	20(17) ↓	↳ 6(7)	↳ 4(5) ↳ 13(16) ↳ 4(5)	7(6) ↓	28(33) ↓	8(7) ↓	↳ 26(33)	34(41) ↓	22(24) ↓	21(27) ↳ 5(6)
2	28(33) ↓						↳ 26(33) ↳ 14(21)			
3					40(43) ↓		↳ 6(7) ↳ 4(5) ↳ 27(38)			
4								45(49) ↓	6(7) ↓	
5				57(65) ↓						
6										
7										
8										
9										
10										
11										



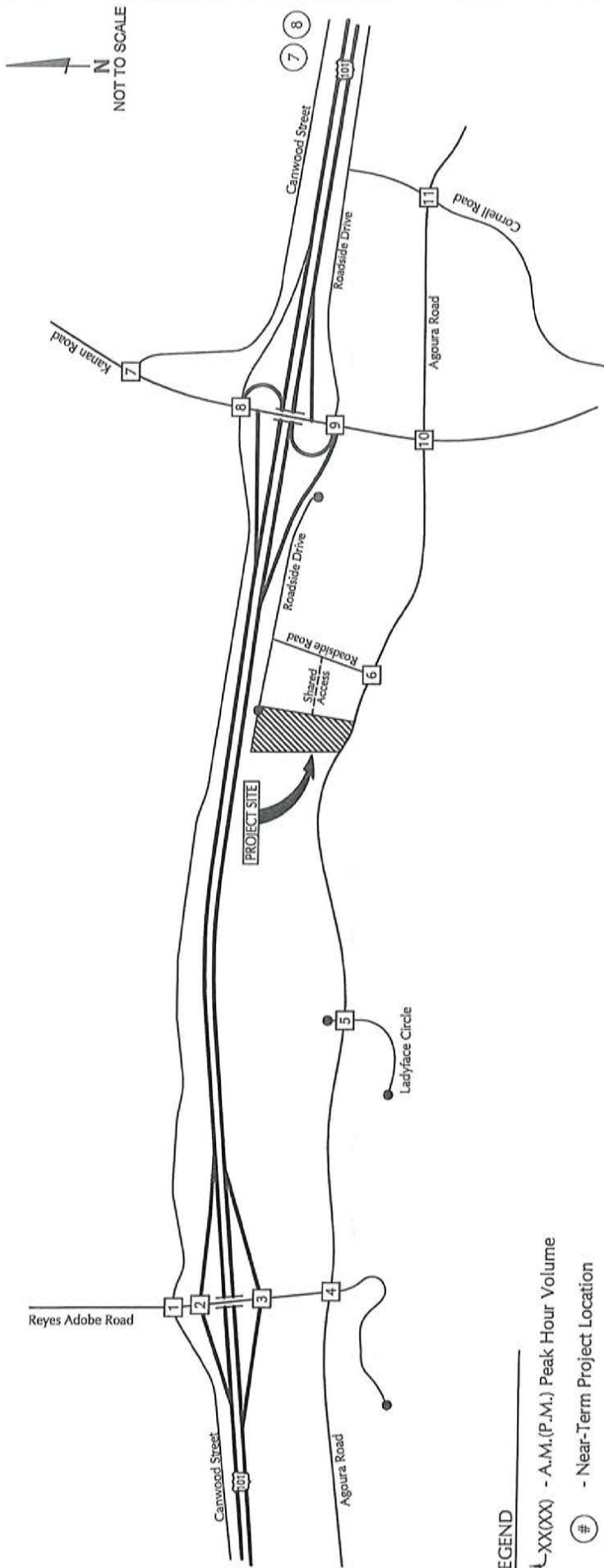
ASSOCIATED  
TRANSPORTATION  
ENGINEERS

NEAR-TERM ADDED TRAFFIC VOLUMES

FIGURE B







LEGEND

XXXXX - A.M.(P.M.) Peak Hour Volume

(#) - Near-Term Project Location

1										
2										
3										
4										
5										
6										
7										
8										
9										
10										
11										



ASSOCIATED  
TRANSPORTATION  
ENGINEERS

NEAR-TERM ADDED TRAFFIC VOLUMES

FIGURE D

MMF - #15068





SIGNAL WARRANT ANALYSIS

Figure 4C-101 (CA), Traffic Signal Warrants Worksheet (Sheet 1 of 5)

COUNT DATE \_\_\_\_\_  
 CALC \_\_\_\_\_ DATE \_\_\_\_\_  
 CHK \_\_\_\_\_ DATE \_\_\_\_\_

DIST \_\_\_\_\_ CO \_\_\_\_\_ RTE \_\_\_\_\_ PM \_\_\_\_\_

Major St: Agoura Road Critical Approach Speed \_\_\_\_\_ mph  
 Minor St: Roadside Road Critical Approach Speed \_\_\_\_\_ mph

Speed limit or critical speed on major street traffic > 40 mph.....  }  
 or } RURAL (R)  
 In built up area of isolated community of < 10,000 population.....  }  
 URBAN (U)

WARRANT 1 - Eight Hour Vehicular Volume SATISFIED YES  NO   
 (Condition A or Condition B or combination of A and B must be satisfied)

Condition A - Minimum Vehicle Volume 100% SATISFIED YES  NO   
 80% SATISFIED YES  NO

APPROACH LANES	MINIMUM REQUIREMENTS (80% SHOWN IN BRACKETS)															
	U	R	U	R												
	1		2 or More													
Both Approaches Major Street	500 (400)	350 (280)	600 (480)	420 (336)	8:00	9:00	10:00	11:00	12:00	1:00	2:00	3:00	4:00	5:00	6:00	Hour
Highest Approach Minor Street	150 (120)	105 (84)	200 (160)	140 (112)	69	29	25	25	32	24	27	34				

Condition B - Interruption of Continuous Traffic 100% SATISFIED YES  NO   
 80% SATISFIED YES  NO

APPROACH LANES	MINIMUM REQUIREMENTS (80% SHOWN IN BRACKETS)															
	U	R	U	R												
	1		2 or More													
Both Approaches Major Street	750 (600)	525 (420)	900 (720)	630 (504)	8:00	9:00	10:00	11:00	12:00	1:00	2:00	3:00	4:00	5:00	6:00	Hour
Highest Approach Minor Street	75 (60)	53 (42)	100 (80)	70 (56)	29	29	25	25	32	24	27	34				

Combination of Conditions A & B SATISFIED YES  NO

REQUIREMENT	CONDITION	✓	FULFILLED
TWO CONDITIONS SATISFIED 80%	A. MINIMUM VEHICULAR VOLUME		Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>
	AND, B. INTERRUPTION OF CONTINUOUS TRAFFIC		
AND, AN ADEQUATE TRIAL OF OTHER ALTERNATIVES THAT COULD CAUSE LESS DELAY AND INCONVENIENCE TO TRAFFIC HAS FAILED TO SOLVE THE TRAFFIC PROBLEMS			Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>

The satisfaction of a traffic signal warrant or warrants shall not in itself require the installation of a traffic control signal.



Figure 4C-101 (CA). Traffic Signal Warrants Worksheet (Sheet 2 of 5)

**WARRANT 2 - Four Hour Vehicular Volume**

SATISFIED\* YES  NO

Record hourly vehicular volumes for any four hours of an average day.

APPROACH LANES	One	2 or More	Hour			
			12:00-1:00	1:00-2:00	2:00-3:00	3:00-4:00
Both Approaches - Major Street	<input checked="" type="checkbox"/>	<input type="checkbox"/>	727	634	658	709
Higher Approach - Minor Street	<input checked="" type="checkbox"/>	<input type="checkbox"/>	25	32	27	34

*All plotted points fall above the applicable curve in Figure 4C-1. (URBAN AREAS)	Yes <input type="checkbox"/>	No <input type="checkbox"/>
<u>OR</u> , All plotted points fall above the applicable curve in Figure 4C-2. (RURAL AREAS)	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>

**WARRANT 3 - Peak Hour**  
 (Part A or Part B must be satisfied)

SATISFIED YES  NO

**PART A**

SATISFIED YES  NO

(All parts 1, 2, and 3 below must be satisfied for the same one hour, for any four consecutive 15-minute periods)

1. The total delay experienced by traffic on one minor street approach (one direction only) controlled by a STOP sign equals or exceeds four vehicle-hours for a one-lane approach, or five vehicle-hours for a two-lane approach; <u>AND</u>	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>
2. The volume on the same minor street approach (one direction only) equals or exceeds 100 vph for one moving lane of traffic or 150 vph for two moving lanes; <u>AND</u>	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>
3. The total entering volume serviced during the hour equals or exceeds 800 vph for intersections with four or more approaches or 650 vph for intersections with three approaches.	Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>

**PART B**

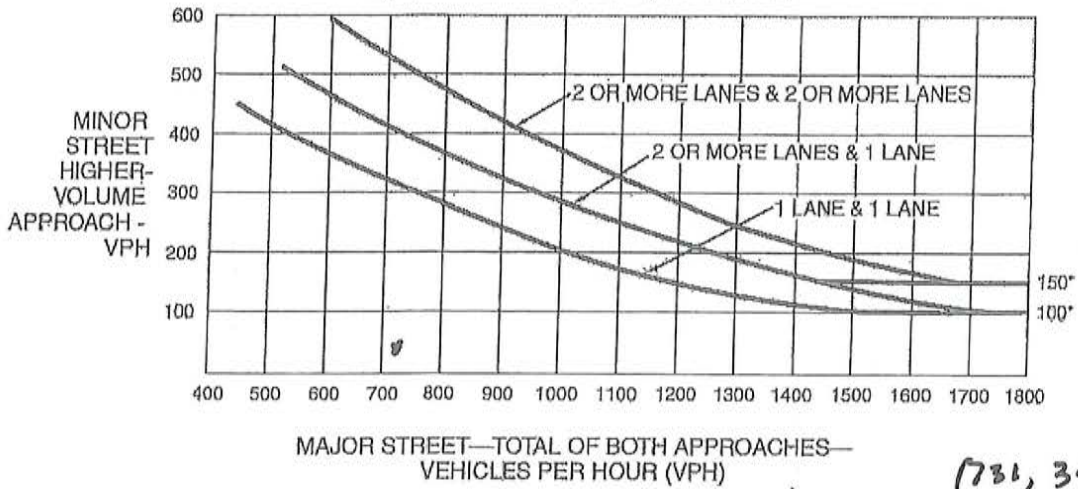
SATISFIED YES  NO

APPROACH LANES	One	2 or More	Hour
Higher Approach - Minor Street	<input checked="" type="checkbox"/>	<input type="checkbox"/>	31

The plotted point falls above the applicable curve in Figure 4C-3. (URBAN AREAS)	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>
<u>OR</u> , The plotted point falls above the applicable curve in Figure 4C-4. (RURAL AREAS)	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>

The satisfaction of a traffic signal warrant or warrants shall not in itself require the installation of a traffic control signal.

Figure 4C-3. Warrant 3, Peak Hour

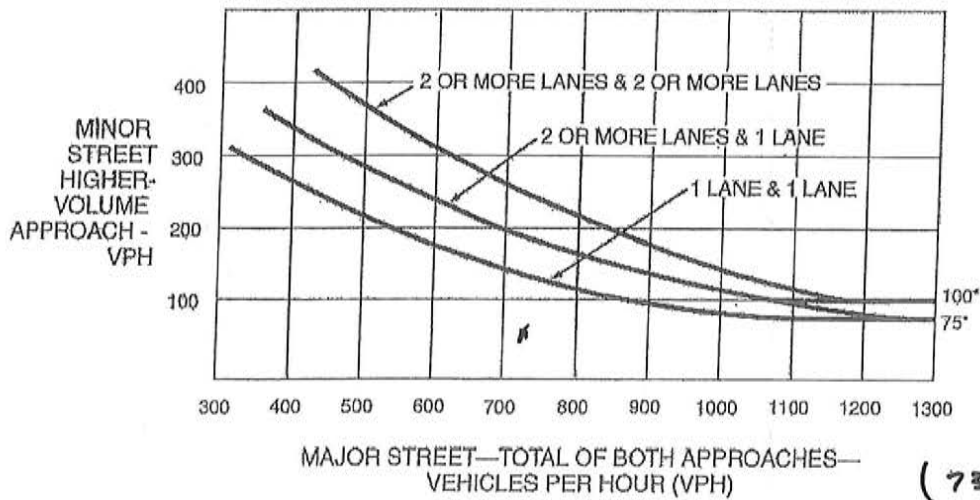


(731, 34)

\*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Figure 4C-4. Warrant 3, Peak Hour (70% Factor)

(COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 MPH ON MAJOR STREET)

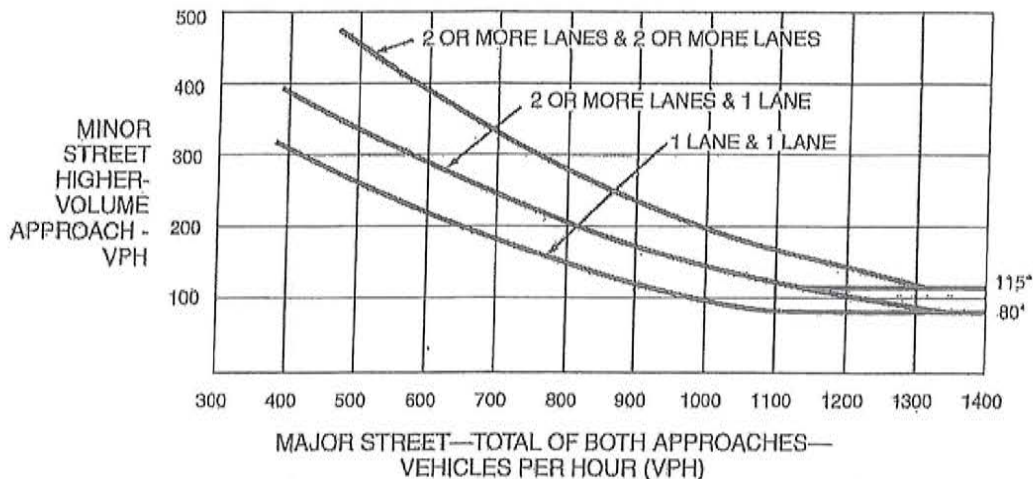


(731, 34)

\*Note: 100 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 75 vph applies as the lower threshold volume for a minor-street approach with one lane.



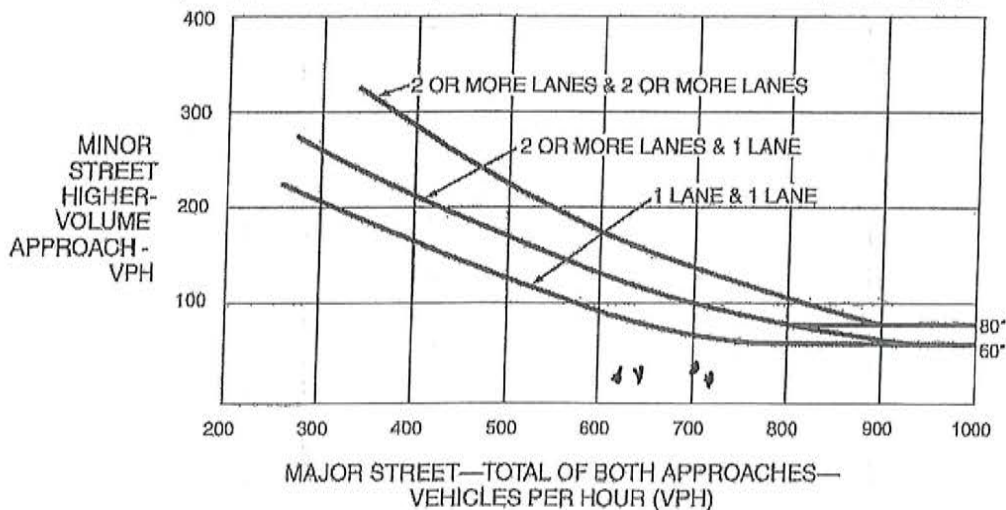
**Figure 4C-1. Warrant 2, Four-Hour Vehicular Volume**



\*Note: 115 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 80 vph applies as the lower threshold volume for a minor-street approach with one lane.

**Figure 4C-2. Warrant 2, Four-Hour Vehicular Volume (70% Factor)**

(COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 MPH ON MAJOR STREET)



\*Note: 80 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 60 vph applies as the lower threshold volume for a minor-street approach with one lane.

Figure 4C-101 (CA). Traffic Signal Warrants Worksheet (Sheet 1 of 5)

COUNT DATE \_\_\_\_\_  
 CALC \_\_\_\_\_ DATE \_\_\_\_\_  
 CHK \_\_\_\_\_ DATE \_\_\_\_\_

DIST \_\_\_\_\_ CO \_\_\_\_\_ RTE \_\_\_\_\_ PM \_\_\_\_\_

Major St: Agave Road Critical Approach Speed \_\_\_\_\_ mph  
 Minor St: Peacocks Road Critical Approach Speed \_\_\_\_\_ mph

Speed limit or critical speed on major street traffic > 40 mph.....  }  
 or  } RURAL (R)  
 In built up area of isolated community of < 10,000 population.....  }  
 } URBAN (U)

**WARRANT 1 - Eight Hour Vehicular Volume** SATISFIED YES  NO   
 (Condition A or Condition B or combination of A and B must be satisfied)

**Condition A - Minimum Vehicle Volume** 100% SATISFIED YES  NO   
 80% SATISFIED YES  NO

APPROACH LANES	MINIMUM REQUIREMENTS (80% SHOWN IN BRACKETS)				Hour																			
	U	R	U	R	8:00	9:00	10:00	11:00	12:00	12:00	1:00	1:00	2:00	2:00	3:00	3:00	4:00	4:00	5:00	5:00	6:00			
Both Approaches Major Street	500 (400)	350 (280)	600 (480)	420 (336)	564	420	583	511	712	622	712	773												
Highest Approach Minor Street	150 (120)	105 (84)	200 (160)	140 (112)	65	54	58	75	79	54	59	72												

**Condition B - Interruption of Continuous Traffic** 100% SATISFIED YES  NO   
 80% SATISFIED YES  NO

APPROACH LANES	MINIMUM REQUIREMENTS (80% SHOWN IN BRACKETS)				Hour																			
	U	R	U	R	8:00	9:00	10:00	11:00	12:00	12:00	1:00	1:00	2:00	2:00	3:00	3:00	4:00	4:00	5:00	5:00	6:00			
Both Approaches Major Street	750 (600)	525 (420)	900 (720)	630 (504)	564	420	583	511	712	622	712	773												
Highest Approach Minor Street	75 (60)	53 (42)	100 (80)	70 (56)	65	54	53	75	79	54	59	72												

**Combination of Conditions A & B** SATISFIED YES  NO

REQUIREMENT	CONDITION	✓	FULFILLED
TWO CONDITIONS SATISFIED 80%	A. MINIMUM VEHICULAR VOLUME		Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>
	AND, B. INTERRUPTION OF CONTINUOUS TRAFFIC		
AND, AN ADEQUATE TRIAL OF OTHER ALTERNATIVES THAT COULD CAUSE LESS DELAY AND INCONVENIENCE TO TRAFFIC HAS FAILED TO SOLVE THE TRAFFIC PROBLEMS			Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>

The satisfaction of a traffic signal warrant or warrants shall not in itself require the installation of a traffic control signal.

Figure 4C-101 (CA). Traffic Signal Warrants Worksheet (Sheet 2 of 5)

**WARRANT 2 - Four Hour Vehicular Volume**

SATISFIED\* YES  NO

Record hourly vehicular volumes for any four hours of an average day.

APPROACH LANES	One	2 or More	Hour			
			12:00-1:00	1:00-2:00	2:00-3:00	3:00-4:00
Both Approaches - Major Street		✓	811	712	712	713
Higher Approach - Minor Street	✓		75	79	59	72

*All plotted points fall above the applicable curve in Figure 4C-1. (URBAN AREAS)	Yes <input type="checkbox"/>	No <input type="checkbox"/>
<u>OR</u> , All plotted points fall above the applicable curve in Figure 4C-2. (RURAL AREAS)	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>

**WARRANT 3 - Peak Hour**  
 (Part A or Part B must be satisfied)

SATISFIED YES  NO

**PART A**

SATISFIED YES  NO

(All parts 1, 2, and 3 below must be satisfied for the same one hour, for any four consecutive 15-minute periods)

1. The total delay experienced by traffic on one minor street approach (one direction only) controlled by a STOP sign equals or exceeds four vehicle-hours for a one-lane approach, or five vehicle-hours for a two-lane approach; <u>AND</u>	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>
2. The volume on the same minor street approach (one direction only) equals or exceeds 100 vph for one moving lane of traffic or 150 vph for two moving lanes; <u>AND</u>	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>
3. The total entering volume serviced during the hour equals or exceeds 800 vph for intersections with four or more approaches or 650 vph for intersections with three approaches.	Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>

**PART B**

SATISFIED YES  NO

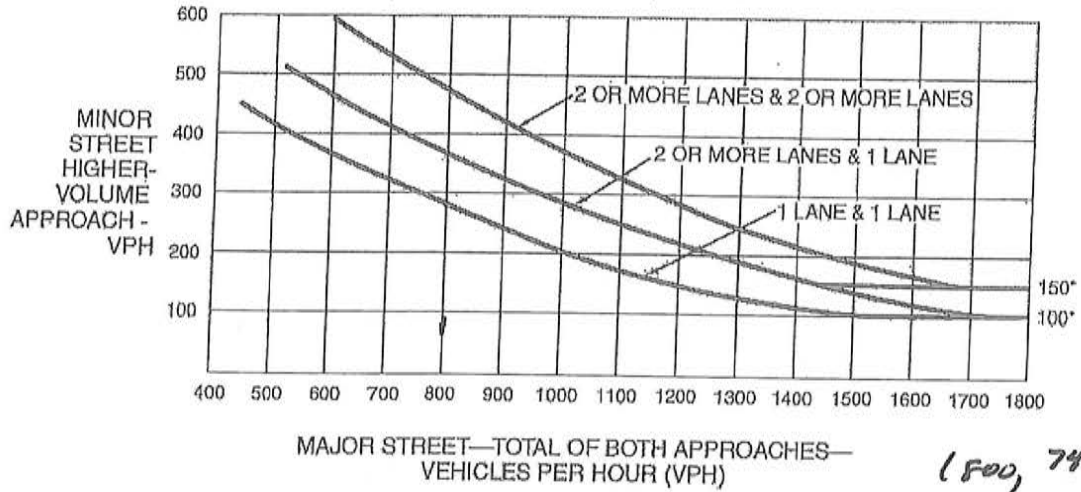
APPROACH LANES	One	2 or More	Hour
Both Approaches - Major Street		✓	800
Higher Approach - Minor Street	✓		74

The plotted point falls above the applicable curve in Figure 4C-3. (URBAN AREAS)	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>
<u>OR</u> , The plotted point falls above the applicable curve in Figure 4C-4. (RURAL AREAS)	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>

The satisfaction of a traffic signal warrant or warrants shall not in itself require the installation of a traffic control signal.



Figure 4C-3. Warrant 3, Peak Hour

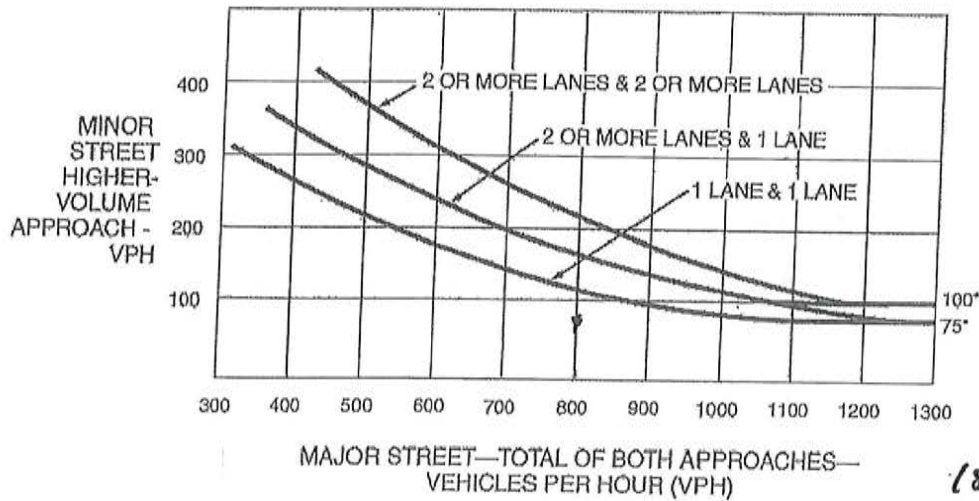


\*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

(800, 74)

Figure 4C-4. Warrant 3, Peak Hour (70% Factor)

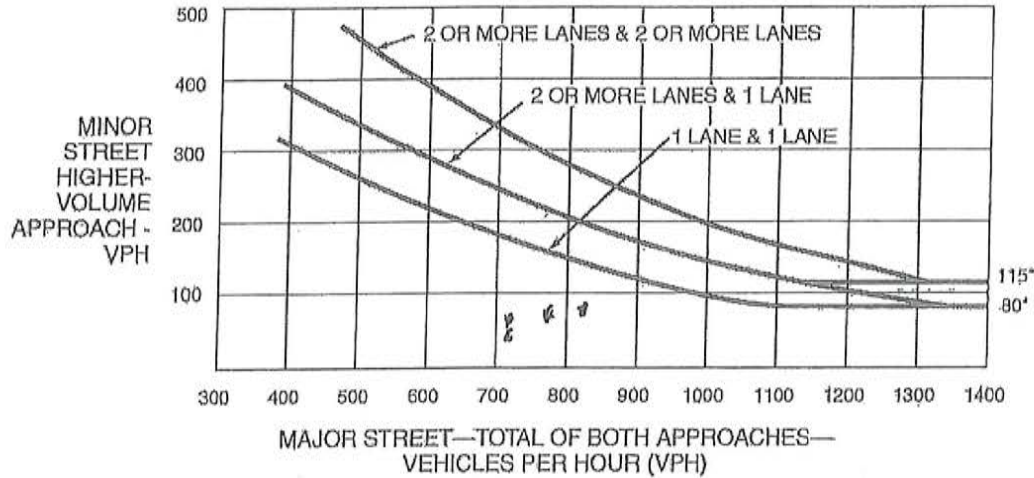
(COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 MPH ON MAJOR STREET)



\*Note: 100 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 75 vph applies as the lower threshold volume for a minor-street approach with one lane.

(800, 74)

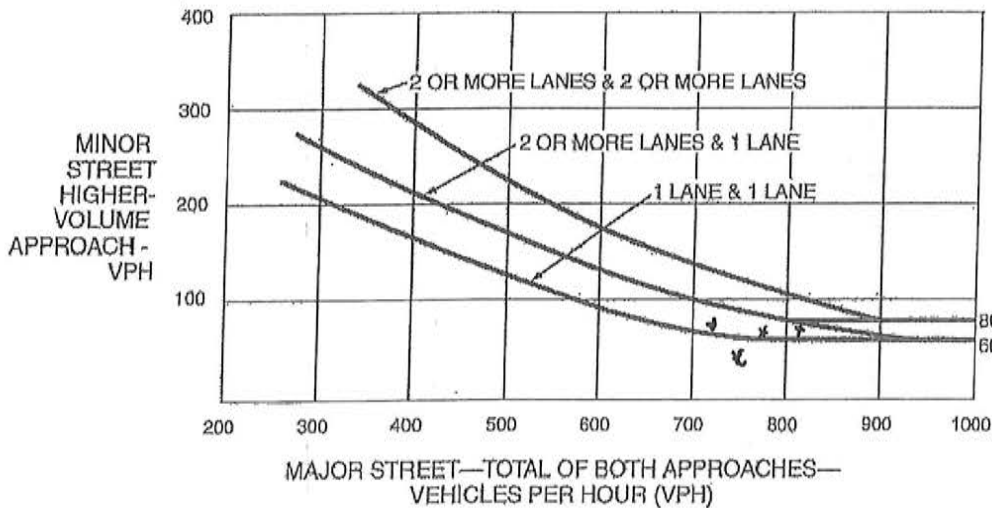
**Figure 4C-1. Warrant 2, Four-Hour Vehicular Volume**



\*Note: 115 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 80 vph applies as the lower threshold volume for a minor-street approach with one lane.

**Figure 4C-2. Warrant 2, Four-Hour Vehicular Volume (70% Factor)**

(COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 MPH ON MAJOR STREET)



\*Note: 80 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 60 vph applies as the lower threshold volume for a minor-street approach with one lane.

Figure 4C-101 (CA), Traffic Signal Warrants Worksheet (Sheet 1 of 5)

COUNT DATE \_\_\_\_\_  
 CALC \_\_\_\_\_ DATE \_\_\_\_\_  
 CHK \_\_\_\_\_ DATE \_\_\_\_\_

DIST \_\_\_\_\_ CO \_\_\_\_\_ RTE \_\_\_\_\_ PM \_\_\_\_\_

Major St: Agave Road Critical Approach Speed 45 mph  
 Minor St: Roadside Road Critical Approach Speed \_\_\_\_\_ mph

Speed limit or critical speed on major street traffic > 40 mph.....  or } RURAL (R)  
 In built up area of isolated community of < 10,000 population.....  }  
 URBAN (U)

WARRANT 1 - Eight Hour Vehicular Volume SATISFIED YES  NO   
 (Condition A or Condition B or combination of A and B must be satisfied)

Condition A - Minimum Vehicle Volume 100% SATISFIED YES  NO   
 80% SATISFIED YES  NO

APPROACH LANES	MINIMUM REQUIREMENTS (80% SHOWN IN BRACKETS)				Hour														
	U		R		U		R		8:00-9:00	9:00-10:00	10:00-11:00	11:00-12:00	12:00-1:00	1:00-2:00	2:00-3:00	3:00-4:00	4:00-5:00	5:00-6:00	6:00-7:00
Both Approaches Major Street	500 (400)	350 (280)	600 (480)	420 (336)	688	587	728	927	834	772	869	920							
Highest Approach Minor Street	150 (120)	105 (84)	200 (160)	140 (112)	94	187	129	195	170	106	102	111							

Condition B - Interruption of Continuous Traffic 100% SATISFIED YES  NO   
 80% SATISFIED YES  NO

APPROACH LANES	MINIMUM REQUIREMENTS (80% SHOWN IN BRACKETS)				Hour														
	U		R		U		R		8:00-9:00	9:00-10:00	10:00-11:00	11:00-12:00	12:00-1:00	1:00-2:00	2:00-3:00	3:00-4:00	4:00-5:00	5:00-6:00	6:00-7:00
Both Approaches Major Street	750 (600)	525 (420)	900 (720)	630 (504)	688	587	728	927	834	772	869	920							
Highest Approach Minor Street	75 (60)	53 (42)	100 (80)	70 (56)	94	187	129	195	170	106	102	111							

Combination of Conditions A & B SATISFIED YES  NO

REQUIREMENT	CONDITION	✓	FULFILLED
TWO CONDITIONS SATISFIED 80%	A. MINIMUM VEHICULAR VOLUME	✓	Yes <input checked="" type="checkbox"/> No <input type="checkbox"/>
	AND, B. INTERRUPTION OF CONTINUOUS TRAFFIC	✓	Yes <input checked="" type="checkbox"/> No <input type="checkbox"/>
AND, AN ADEQUATE TRIAL OF OTHER ALTERNATIVES THAT COULD CAUSE LESS DELAY AND INCONVENIENCE TO TRAFFIC HAS FAILED TO SOLVE THE TRAFFIC PROBLEMS			Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>

The satisfaction of a traffic signal warrant or warrants shall not in itself require the installation of a traffic control signal.



Figure 4C-101 (CA). Traffic Signal Warrants Worksheet (Sheet 2 of 5)

**WARRANT 2 - Four Hour Vehicular Volume**

SATISFIED\* YES  NO

Record hourly vehicular volumes for any four hours of an average day.

APPROACH LANES			12:00	1:00	2:00	3:00	Hour
	One	2 or More					
Both Approaches - Major Street		<input checked="" type="checkbox"/>	927	834	864	920	
Higher Approach - Minor Street	<input checked="" type="checkbox"/>		195	170	102	111	

*All plotted points fall above the applicable curve in Figure 4C-1. (URBAN AREAS)	Yes <input type="checkbox"/>	No <input type="checkbox"/>
<u>OR</u> , All plotted points fall above the applicable curve in Figure 4C-2. (RURAL AREAS)	Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>

**WARRANT 3 - Peak Hour**  
 (Part A or Part B must be satisfied)

SATISFIED YES  NO

**PART A**

SATISFIED YES  NO

(All parts 1, 2, and 3 below must be satisfied for the same one hour, for any four consecutive 15-minute periods)

1. The total delay experienced by traffic on one minor street approach (one direction only) controlled by a STOP sign equals or exceeds four vehicle-hours for a one-lane approach, or five vehicle-hours for a two-lane approach; <u>AND</u>	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>
2. The volume on the same minor street approach (one direction only) equals or exceeds 100 vph for one moving lane of traffic or 150 vph for two moving lanes; <u>AND</u>	Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>
3. The total entering volume serviced during the hour equals or exceeds 800 vph for intersections with four or more approaches or 650 vph for intersections with three approaches.	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>

**PART B**

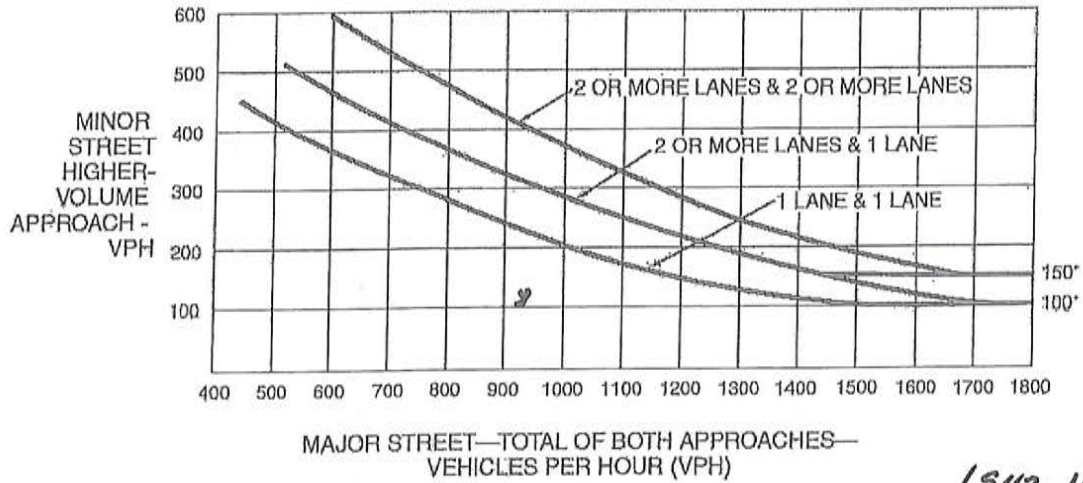
SATISFIED YES  NO

APPROACH LANES			Hour
	One	2 or More	
Both Approaches - Major Street		<input checked="" type="checkbox"/>	942
Higher Approach - Minor Street	<input checked="" type="checkbox"/>		112

The plotted point falls above the applicable curve in Figure 4C-3. (URBAN AREAS)	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>
<u>OR</u> , The plotted point falls above the applicable curve in Figure 4C-4. (RURAL AREAS)	Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>

The satisfaction of a traffic signal warrant or warrants shall not in itself require the installation of a traffic control signal.

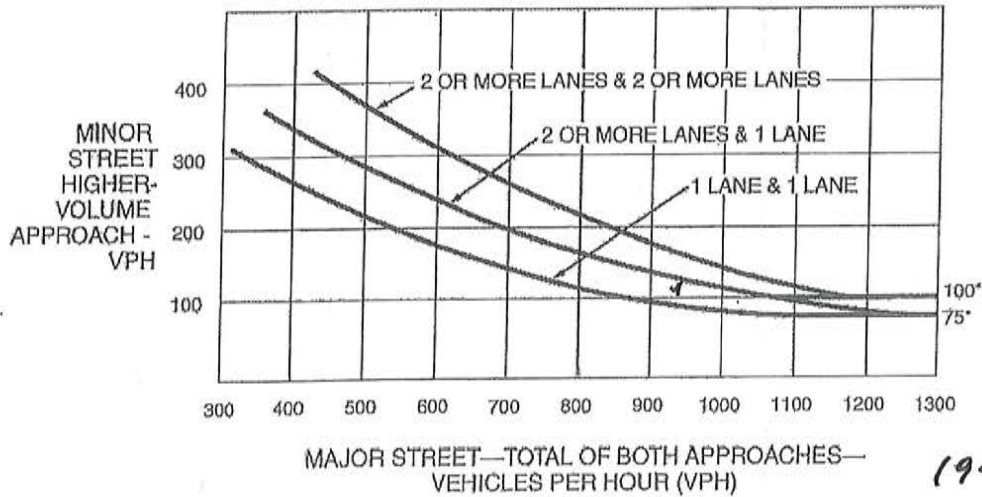
Figure 4C-3. Warrant 3, Peak Hour



\*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

(942, 112)

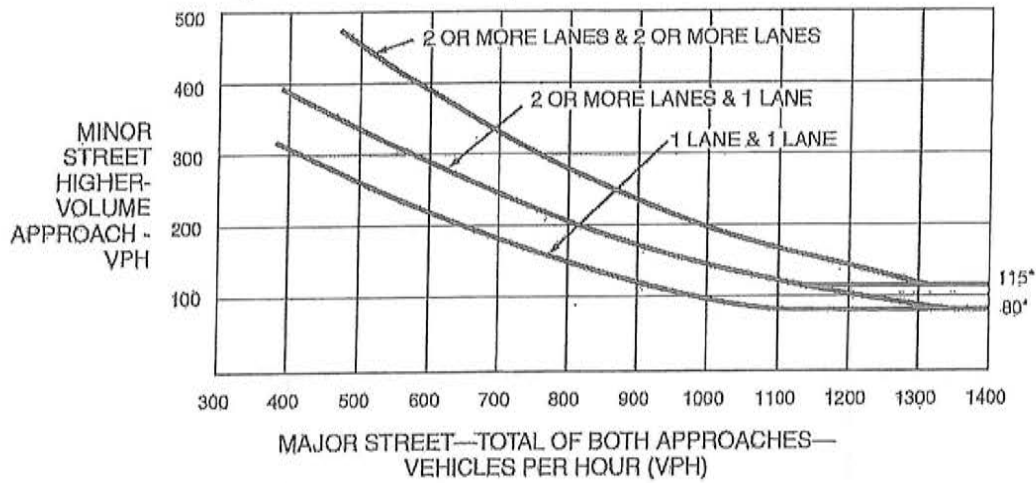
Figure 4C-4. Warrant 3, Peak Hour (70% Factor)  
(COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 MPH ON MAJOR STREET)



\*Note: 100 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 75 vph applies as the lower threshold volume for a minor-street approach with one lane.

(942, 112)

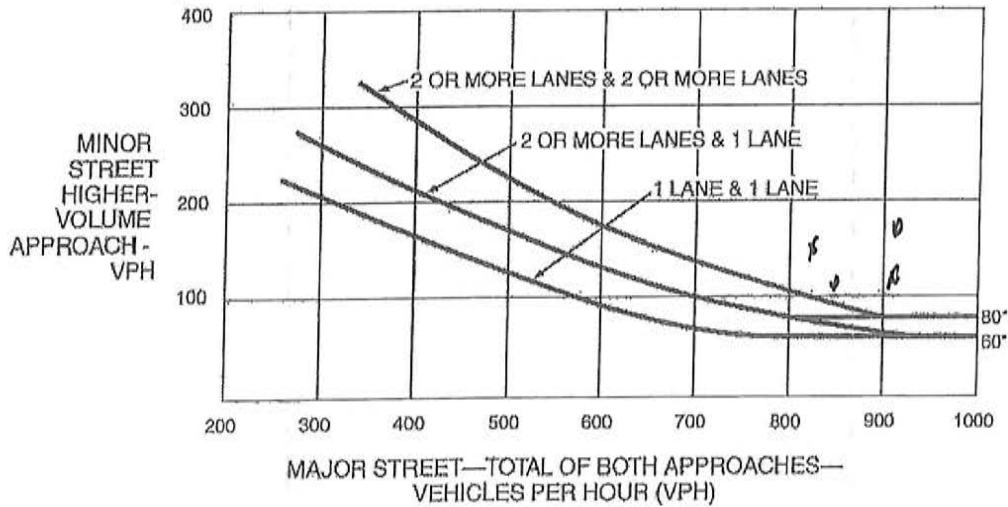
**Figure 4C-1. Warrant 2, Four-Hour Vehicular Volume**



\*Note: 115 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 80 vph applies as the lower threshold volume for a minor-street approach with one lane.

**Figure 4C-2. Warrant 2, Four-Hour Vehicular Volume (70% Factor)**

(COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 MPH ON MAJOR STREET)



\*Note: 80 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 60 vph applies as the lower threshold volume for a minor-street approach with one lane.



Figure 4C-101 (CA). Traffic Signal Warrants Worksheet (Sheet 1 of 5)

COUNT DATE \_\_\_\_\_  
 CALC \_\_\_\_\_ DATE \_\_\_\_\_  
 CHK \_\_\_\_\_ DATE \_\_\_\_\_

DIST \_\_\_\_\_ CO \_\_\_\_\_ RTE \_\_\_\_\_ PM \_\_\_\_\_

Major St: Agave Road Critical Approach Speed 45 mph  
 Minor St: Roadside Road Critical Approach Speed \_\_\_\_\_ mph

Speed limit or critical speed on major street traffic > 40 mph.....  or  } RURAL (R)  
 In built up area of isolated community of < 10,000 population.....  } URBAN (U)

**WARRANT 1 - Eight Hour Vehicular Volume** SATISFIED YES  NO   
 (Condition A or Condition B or combination of A and B must be satisfied)

**Condition A - Minimum Vehicle Volume** 100% SATISFIED YES  NO   
 80% SATISFIED YES  NO

APPROACH LANES	MINIMUM REQUIREMENTS (80% SHOWN IN BRACKETS)				Hour											
	1		2 or More		8:00	9:00	10:00	11:00	12:00	1:00	2:00	3:00	4:00	5:00	6:00	7:00
Both Approaches Major Street	500 (400)	350 (280)	600 (480)	420 (336)	749	628	761	977	881	802	901	958				
Highest Approach Minor Street	150 (120)	105 (84)	200 (160)	140 (112)	130	212	162	245	217	136	134	149				

**Condition B - Interruption of Continuous Traffic** 100% SATISFIED YES  NO

80% SATISFIED YES  NO

APPROACH LANES	MINIMUM REQUIREMENTS (80% SHOWN IN BRACKETS)				Hour											
	1		2 or More		8:00	9:00	10:00	11:00	12:00	1:00	2:00	3:00	4:00	5:00	6:00	7:00
Both Approaches Major Street	750 (600)	525 (420)	900 (720)	630 (504)	748	628	761	977	881	802	901	958				
Highest Approach Minor Street	75 (60)	53 (42)	100 (80)	70 (56)	130	212	162	245	217	136	134	149				

**Combination of Conditions A & B** SATISFIED YES  NO

REQUIREMENT	CONDITION	✓	FULFILLED
TWO CONDITIONS SATISFIED 80%	A. MINIMUM VEHICULAR VOLUME	✓	Yes <input checked="" type="checkbox"/> No <input type="checkbox"/>
	AND, B. INTERRUPTION OF CONTINUOUS TRAFFIC	✓	
AND, AN ADEQUATE TRIAL OF OTHER ALTERNATIVES THAT COULD CAUSE LESS DELAY AND INCONVENIENCE TO TRAFFIC HAS FAILED TO SOLVE THE TRAFFIC PROBLEMS			Yes <input type="checkbox"/> No <input checked="" type="checkbox"/>

The satisfaction of a traffic signal warrant or warrants shall not in itself require the installation of a traffic control signal.

Figure 4C-101 (CA). Traffic Signal Warrants Worksheet (Sheet 2 of 5)

**WARRANT 2 - Four Hour Vehicular Volume**

SATISFIED\* YES  NO

Record hourly vehicular volumes for any four hours of an average day.

APPROACH LANES	2 or One More		12:00 1:00	1:00 2:00	2:00 3:00	3:00 4:00	4:00 5:00	5:00 6:00	Hour
	One	More							
Both Approaches - Major Street		<input checked="" type="checkbox"/>	577	881	901	958			
Higher Approach - Minor Street	<input checked="" type="checkbox"/>		245	217	132	149			

*All plotted points fall above the applicable curve in Figure 4C-1. (URBAN AREAS)	Yes <input type="checkbox"/>	No <input type="checkbox"/>
<u>OR</u> , All plotted points fall above the applicable curve in Figure 4C-2. (RURAL AREAS)	Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>

**WARRANT 3 - Peak Hour**  
 (Part A or Part B must be satisfied)

SATISFIED YES  NO

**PART A**

SATISFIED YES  NO

(All parts 1, 2, and 3 below must be satisfied for the same one hour, for any four consecutive 15-minute periods)

1. The total delay experienced by traffic on one minor street approach (one direction only) controlled by a STOP sign equals or exceeds four vehicle-hours for a one-lane approach, or five vehicle-hours for a two-lane approach; <u>AND</u>	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>
2. The volume on the same minor street approach (one direction only) equals or exceeds 100 vph for one moving lane of traffic or 150 vph for two moving lanes; <u>AND</u>	Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>
3. The total entering volume serviced during the hour equals or exceeds 800 vph for intersections with four or more approaches or 650 vph for intersections with three approaches.	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>

**PART B**

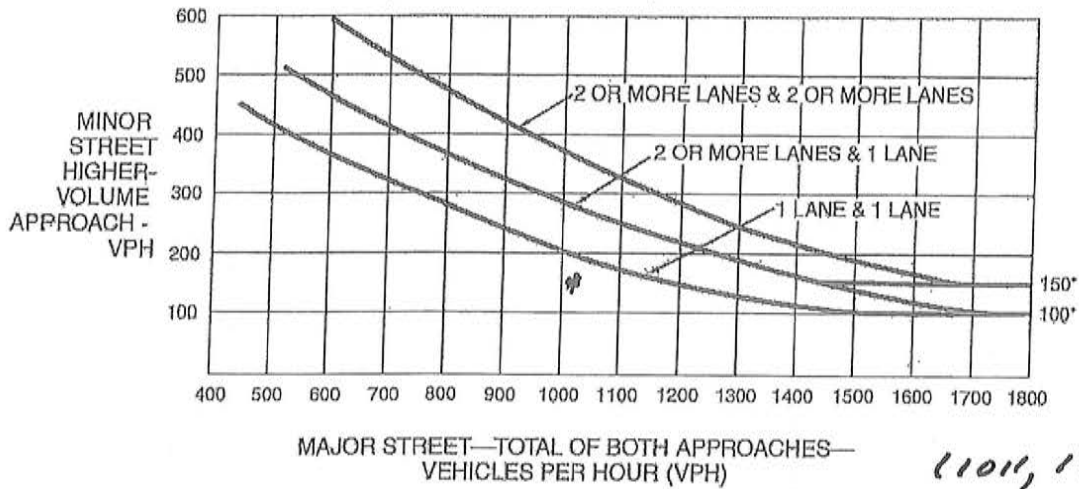
SATISFIED YES  NO

APPROACH LANES	2 or One More		Hour
	One	More	
Both Approaches - Major Street		<input checked="" type="checkbox"/>	1011
Higher Approach - Minor Street	<input checked="" type="checkbox"/>		152

The plotted point falls above the applicable curve in Figure 4C-3. (URBAN AREAS)	Yes <input type="checkbox"/>	No <input checked="" type="checkbox"/>
<u>OR</u> , The plotted point falls above the applicable curve in Figure 4C-4. (RURAL AREAS)	Yes <input checked="" type="checkbox"/>	No <input type="checkbox"/>

The satisfaction of a traffic signal warrant or warrants shall not in itself require the installation of a traffic control signal.

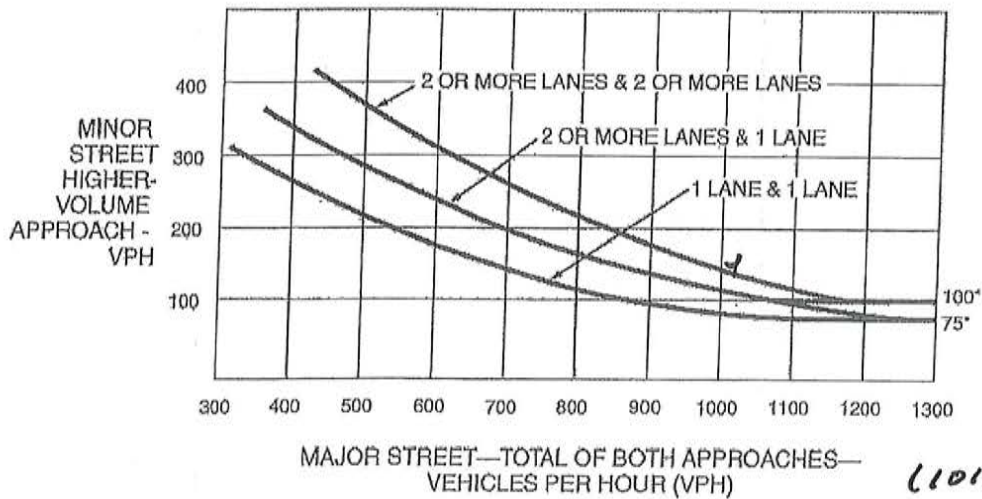
Figure 4C-3. Warrant 3, Peak Hour



\*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Figure 4C-4. Warrant 3, Peak Hour (70% Factor)

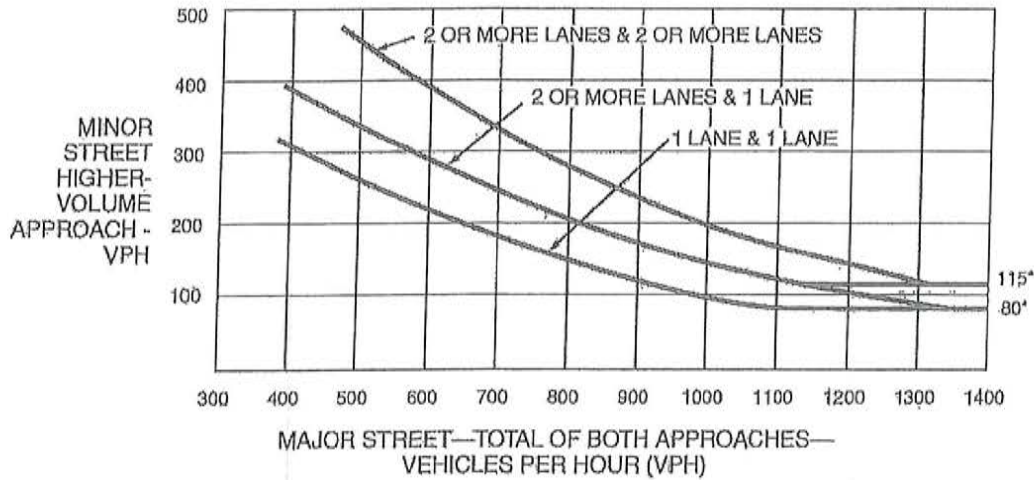
(COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 MPH ON MAJOR STREET)



\*Note: 100 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 75 vph applies as the lower threshold volume for a minor-street approach with one lane.



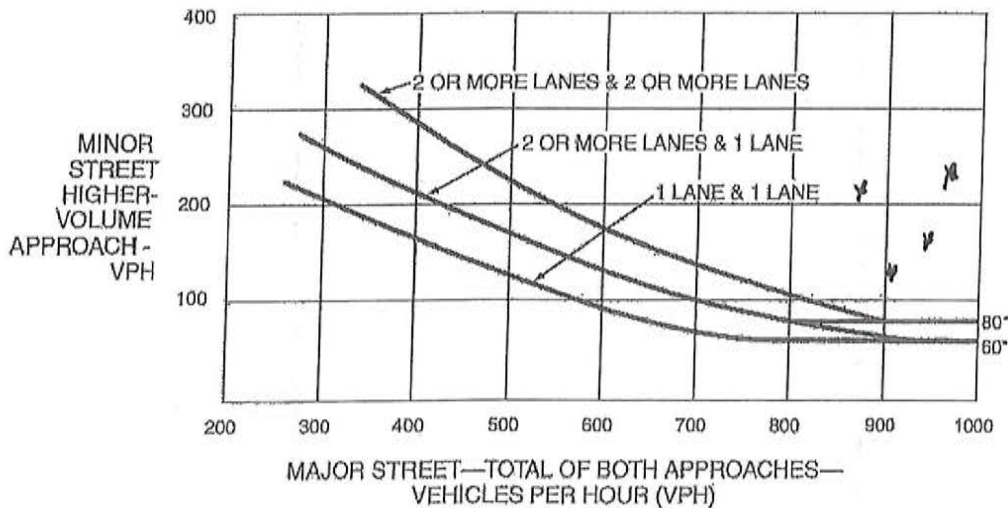
**Figure 4C-1. Warrant 2, Four-Hour Vehicular Volume**



\*Note: 115 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 80 vph applies as the lower threshold volume for a minor-street approach with one lane.

**Figure 4C-2. Warrant 2, Four-Hour Vehicular Volume (70% Factor)**

(COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 MPH ON MAJOR STREET)



\*Note: 80 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 60 vph applies as the lower threshold volume for a minor-street approach with one lane.