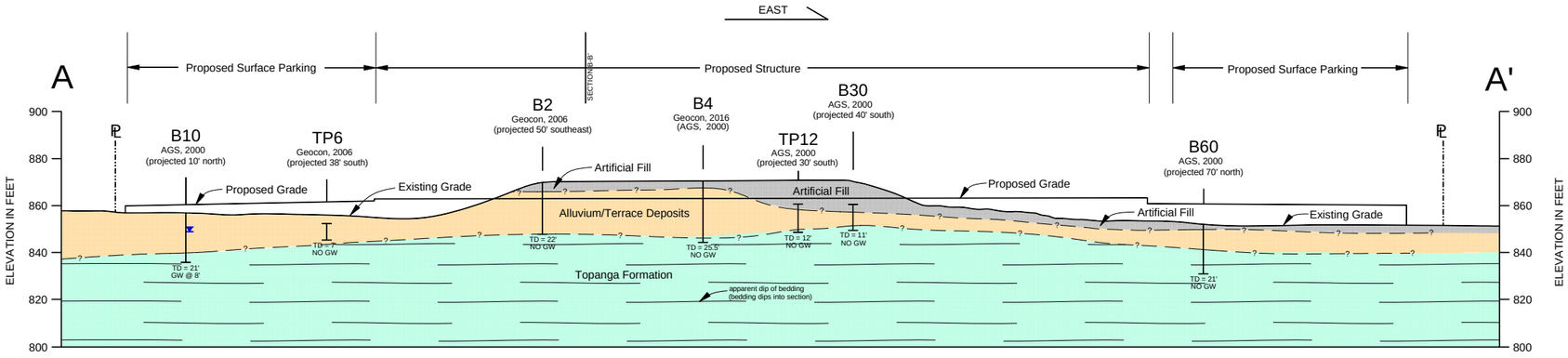

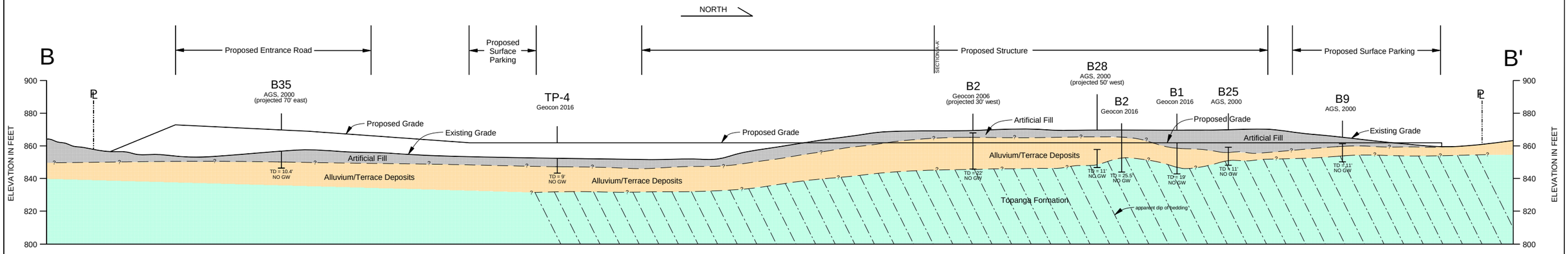

NOTES:

1. THE SECTIONS ARE BASED ON GEOLOGIC CONDITIONS AT BORING LOCATIONS AND AT SURFACE EXPOSURES MAPPED DURING THE INVESTIGATION. THE GEOLOGIC CONDITIONS BETWEEN SUCH LOCATIONS HAVE BEEN INTERPOLATED. LOCALIZED VARIATIONS COULD OCCUR. THE SECTIONS ARE INTENDED FOR DESCRIPTIVE PURPOSES ONLY.
2. SEE FIGURE 1 FOR LOCATION OF SECTIONS.

LEGEND

- Artificial Fill
- Alluvium/Terrace Deposits
- Topanga Formation
- Geologic Contact
- Groundwater Level Encountered in Boring

SCALE: 1" = 20'
(HORIZONTAL AND VERTICAL SCALE)

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DRAFTED BY: SJB | CHECKED BY: SPK

GEOLOGIC SECTION

AGOURA HILLS HHG HOTEL DEVELOPMENT LP
29508 ROADSIDE DRIVE
AGOURA HILLS, CALIFORNIA

FEB. 2016 | PROJECT NO. A8487-06-04 | FIGURE 2

COLUMN SETTLEMENT CALCULATIONS

(AT-GRADE FOOTINGS-SEE INSTRUCTIONS BELOW)

CLIENT: 29508 Roadside Drive F.F. ELEV = 863 FT
FILE NUMBER: A8487-06-04

COLUMN LOAD	602	(KIPS)
DESIGN BEARING VALUE	3000	(PSF)
DEAD LOAD	75	(PERCENT)
LIVE LOAD	25	(PERCENT)
FOOTING DEPTH (DF)	2	(FEET)
SOIL DENSITY (G)	120	(PCF)
FOOTING SIZE(SQUARE)(a)	14.2	(FEET)
REAL LOAD (PR)	3000	(PSF)

Di = INITIAL DEPTH OF SLICE (FEET)
Df = FINAL DEPTH OF SLICE (FEET)
D1 = AVG. DEPTH OF SLICE BELOW ORIG. GRADE (FEET)
D2 = AVG. DEPTH BELOW FOOTING (FEET)
PV = VERTICAL PRESSURE (PERCENTAGE OF REAL LOAD)

ENTER THE PERCENTAGE OF CONSOLIDATION FROM PLATE C AT THE INITIAL TO FINAL PRESSURES.

Di	Df	ELEVATION FTG. BOT. (FT)	D1	(z) D2	a/z	PV	SOIL PRESSURES (KIPS) INITIAL	FINAL	PERCENT CONSOL.	SLICE THICKNESS (INCHES)	SETTLE. (INCHES)	SAMPLE
2	3	860	3	1	28.3	97.0	0.3	3.2	0.9	12	0.11	B8 @ 0-10'
3	4	859	4	2	9.4	88.0	0.4	3.1	0.8	12	0.10	B8 @ 0-10'
4	6	857	5	3	4.7	75.0	0.6	2.9	0.7	24	0.17	B8 @ 0-10'
6	8	855	7	5	2.8	61.0	0.8	2.7	1.0	24	0.24	B11 @ 2'
8	10	853	9	7	2.0	49.0	1.1	2.6	0.8	24	0.19	B11 @ 2'
10	12	851	11	9	1.6	39.0	1.3	2.5	0.6	24	0.14	B9 @ 5'
12	14	849	13	11	1.3	33.0	1.6	2.6	0.5	24	0.12	B11 @ 7'
14	16	847	15	13	1.1	26.0	1.8	2.6	0.3	24	0.07	B11 @ 7'
16	18	845	17	15	0.9	22.0	2.0	2.7	0.2	24	0.05	B11 @ 7'
18	20	843	19	17	0.8	20.0	2.3	2.9	0.1	24	0.02	B11 @ 7'
20	22	841	21	19	0.7	17.0	2.5	3.0	0.1	24	0.02	B11 @ 7'
22	24	839	23	21	0.7	15.0	2.8	3.2	0.0	24	0.00	Bedrock
24	26	837	25	23	0.6	13.0	3.0	3.4	0.0	24	0.00	Bedrock
26	28	835	27	25	0.6	12.0	3.2	3.6	0.0	24	0.00	Bedrock
28	30	833	29	27	0.5	9.0	3.5	3.8	0.0	24	0.00	Bedrock
30	32	831	31	29	0.5	0.0	3.7	3.7	0.0	24	0.00	Bedrock
32	34	829	33	31	0.5	0.0	4.0	4.0	0.0	24	0.00	Bedrock
34	36	827	35	33	0.4	0.0	4.2	4.2	0.0	24	0.00	Bedrock
36	38	825	37	35	0.4	0.0	4.4	4.4	0.0	24	0.00	Bedrock
38	40	823	39	37	0.4	0.0	4.7	4.7	0.0	24	0.00	Bedrock
40	42	821	41	39	0.4	0.0	4.9	4.9	0.0	24	0.00	Bedrock
									TOTAL SETTLEMENT		1.24	INCHES

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Drafted by: RDG | Checked by: HHD

COLUMN SETTLEMENT

AGOURA HILLS HHG HOTEL DEVELOPMENT, LP.
29508 ROADSIDE DRIVE
AGOURA HILLS, CALIFORNIA

FEB. 2016 | PROJECT NO. A8487-06-04 | FIG. 3

WALL SETTLEMENT CALCULATIONS

(AT-GRADE FOOTINGS-SEE INSTRUCTIONS BELOW)

PROJECT NAME: 29508 Roadside Drive
Project # A8487-06-04

WALL LOAD	8	(KIPS/FOOT)
DESIGN BEARING VALUE	3000	(PSF)
DEAD LOAD	75	(PERCENT)
LIVE LOAD	25	(PERCENT)
FOOTING DEPTH (DF)	2	(FEET)
SOIL DENSITY (G)	120	(PCF)
FOOTING SIZE(WIDTH)	2.7	(FEET)
REAL LOAD (PR)	3000	(PSF)

Di = INITIAL DEPTH OF SLICE (FEET)
Df = FINAL DEPTH OF SLICE (FEET)
D1 = AVG. DEPTH OF SLICE BELOW ORIG. GRADE (FEET)
D2 = AVG. DEPTH BELOW FOOTING (FEET)
PV = VERTICAL PRESSURE (PERCENTAGE OF REAL LOAD)

ENTER THE PERCENTAGE OF CONSOLIDATION FROM PLATE C AT THE INITIAL TO FINAL PRESSURES.

Finish floor elevation: 863 FEET

Di	Df	D1	D2	Elevation	b/z	SOIL PRESSURES (KIPS) PV	INITIAL	FINAL	PERCENT CONSOL.	SLICE THICKNESS (INCHES)	SETTLEMENT (INCHES)	SAMPLE
2	3	2.5	0.5	861	5.3	85	0.3	2.9	1.0	12	0.12	B8 @ 0-10'
3	4	3.5	1.5	860	1.8	60	0.4	2.2	0.8	12	0.10	B8 @ 0-10'
4	5	4.5	2.5	859	1.1	44	0.5	1.9	0.6	12	0.07	B8 @ 0-10'
5	7	6	4	858	0.7	31	0.7	1.7	0.4	24	0.10	B8 @ 0-10'
7	9	8	6	856	0.4	22	1.0	1.6	0.3	24	0.07	B7 @ 10'
9	11	10	8	854	0.3	16	1.2	1.7	0.2	24	0.05	B11 @ 2'
11	13	12	10	852	0.3	14	1.4	1.9	0.1	24	0.02	B11 @ 2'
13	15	14	12	850	0.2	12	1.7	2.0	0.1	24	0.02	B9 @ 5'
15	17	16	14	848	0.2	10	1.9	2.2	0.0	24	0.00	B11 @ 7'
17	19	18	16	846	0.2	8	2.2	2.4	0.0	24	0.00	B11 @ 7'
19	21	20	18	844	0.1	7	2.4	2.6	0.0	24	0.00	B11 @7'
21	23	22	20	842	0.1	0	2.6	2.6	0.0	24	0.00	B11 @ 7'
23	25	24	22	840	0.1	0	2.9	2.9	0.0	24	0.00	Bedrock
25	27	26	24	838	0.1	0	3.1	3.1	0.0	24	0.00	Bedrock
27	29	28	26	836	0.1	0	3.4	3.4	0.0	24	0.00	Bedrock
29	31	30	28	834	0.1	0	3.6	3.6	0.0	24	0.00	Bedrock
31	33	32	30	832	0.1	0	3.8	3.8	0.0	24	0.00	Bedrock
33	35	34	32	830	0.1	0	4.1	4.1	0.0	24	0.00	Bedrock
35	37	36	34	828	0.1	0	4.3	4.3	0.0	24	0.00	Bedrock
								TOTAL SETTLEMENT			0.552	INCH

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WALL SETTLEMENT

AGOURA HILLS HHG HOTEL DEVELOPMENT, LP.
29508 ROADSIDE DRIVE
AGOURA HILLS, CALIFORNIA

FEB. 2016 | PROJECT NO. A8487-06-04 | FIG. 4

NATIVE SOIL SETTLEMENT CALCULATIONS WITH 10 FT OF ENGINEERED FILL

(AT-GRADE FOOTINGS-SEE INSTRUCTIONS BELOW)

CLIENT: 29508 Roadside Drive F.F. ELEV = 853 FT
FILE NUMBER: A8487-06-04

FILL LOAD	2.0	(KIPS)
DESIGN BEARING VALUE	1200	(PSF)
DEAD LOAD	75	(PERCENT)
LIVE LOAD	25	(PERCENT)
FOOTING DEPTH (DF)	0	(FEET)
SOIL DENSITY (G)	120	(PCF)
FOOTING SIZE(SQUARE)(a)	1.3	(FEET)
REAL LOAD (PR)	1200	(PSF)

Di = INITIAL DEPTH OF SLICE (FEET)
Df = FINAL DEPTH OF SLICE (FEET)
D1 = AVG. DEPTH OF SLICE BELOW ORIG. GRADE (FEET)
D2 = AVG. DEPTH BELOW FOOTING (FEET)
PV = VERTICAL PRESSURE (PERCENTAGE OF REAL LOAD)

ENTER THE PERCENTAGE OF CONSOLIDATION FROM PLATE C AT THE INITIAL TO FINAL PRESSURES.

Di	Df	ELEVATION FTG. BOT. (FT)	D1	(z) D2	a/z	PV	SOIL PRESSURES (KIPS) INITIAL	FINAL	PERCENT CONSOL.	SLICE THICKNESS (INCHES)	SETTLE. (INCHES)	SAMPLE
0	1	852	1	1	2.6	61.0	0.1	0.8	1.2	12	0.14	B9 @ 5'
1	2	851	2	2	0.9	22.0	0.2	0.4	0.2	12	0.02	B9 @ 5'
2	4	849	3	3	0.4	7.0	0.4	0.4	0.0	24	0.00	B11 @ 7'
4	6	847	5	5	0.3	0.0	0.6	0.6	0.0	24	0.00	B11 @ 7'
6	8	845	7	7	0.2	0.0	0.8	0.8	0.0	24	0.00	B11 @ 7'
8	10	843	9	9	0.1	0.0	1.1	1.1	0.0	24	0.00	B11 @ 7'
10	12	841	11	11	0.1	0.0	1.3	1.3	0.0	24	0.00	B11 @ 7'
12	14	839	13	13	0.1	0.0	1.6	1.6	0.0	24	0.00	Bedrock
14	16	837	15	15	0.1	0.0	1.8	1.8	0.0	24	0.00	Bedrock
16	18	835	17	17	0.1	0.0	2.0	2.0	0.0	24	0.00	Bedrock
18	20	833	19	19	0.1	0.0	2.3	2.3	0.0	24	0.00	Bedrock
20	22	831	21	21	0.1	0.0	2.5	2.5	0.0	24	0.00	Bedrock
22	24	829	23	23	0.1	0.0	2.8	2.8	0.0	24	0.00	Bedrock
24	26	827	25	25	0.1	0.0	3.0	3.0	0.0	24	0.00	Bedrock
26	28	825	27	27	0.0	0.0	3.2	3.2	0.0	24	0.00	Bedrock
28	30	823	29	29	0.0	0.0	3.5	3.5	0.0	24	0.00	Bedrock
30	32	821	31	31	0.0	0.0	3.7	3.7	0.0	24	0.00	Bedrock
32	34	819	33	33	0.0	0.0	4.0	4.0	0.0	24	0.00	Bedrock
34	36	817	35	35	0.0	0.0	4.2	4.2	0.0	24	0.00	Bedrock
36	38	815	37	37	0.0	0.0	4.4	4.4	0.0	24	0.00	Bedrock
38	40	813	39	39	0.0	0.0	4.7	4.7	0.0	24	0.00	Bedrock
									TOTAL SETTLEMENT		0.17	INCHES

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ENGINEERED FILL SETTLEMENT

AGOURA HILLS HHG HOTEL DEVELOPMENT, LP.
29508 ROADSIDE DRIVE
AGOURA HILLS, CALIFORNIA

FEB. 2016 | PROJECT NO. A8487-06-04 | FIG. 5

PROJECT NO. A8487-06-04

BORING 5

ELEV. (MSL.) **871** DATE COMPLETED **1/11/16**

EQUIPMENT **HOLLOW STEM AUGER** BY: **RDG**

DEPTH IN FEET	SAMPLE NO.	LITHOLOGY	GROUNDWATER	SOIL CLASS (USCS)	MATERIAL DESCRIPTION	PENETRATION RESISTANCE (BLOWS/FT*)	DRY DENSITY (P.C.F.)	MOISTURE CONTENT (%)
0	BULK 0-9'				**ARTIFICIAL FILL** Sandy Clay, soft, wet, brown, fine- to medium-grained, trace coarse-grained, trace fine gravel. - firm, slightly moist, fine- to medium-grained			
2								
4								
6					**BEDROCK (TOPANGA FORMATION)** Siltstone and Claystone, soft, dark brown and yellowish brown, fine-grained, thinly bedded, moderately weathered, moderately fractured.			
8								
10	B1@10'					50 (5")	105.5	16.9
					Total depth of boring: 10½ feet Fill to 4½ feet. No groundwater encountered. Backfilled with soil cuttings and tamped. *Penetration resistance for 140-pound hammer falling 30 inches by auto hammer.			

A8487-06-04 BORING LOGS.GPJ

Figure A1,
Log of Boring 5, Page 1 of 1

SAMPLE SYMBOLS

- ☐ ... SAMPLING UNSUCCESSFUL
- ◧ ... STANDARD PENETRATION TEST
- ■ ... DRIVE SAMPLE (UNDISTURBED)
- ⊠ ... DISTURBED OR BAG SAMPLE
- ◪ ... CHUNK SAMPLE
- ▼ ... WATER TABLE OR SEEPAGE

NOTE: THE LOG OF SUBSURFACE CONDITIONS SHOWN HEREON APPLIES ONLY AT THE SPECIFIC BORING OR TRENCH LOCATION AND AT THE DATE INDICATED. IT IS NOT WARRANTED TO BE REPRESENTATIVE OF SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND TIMES.

GEOCON

PROJECT NO. A8487-06-04

BORING 6

ELEV. (MSL.) **869** DATE COMPLETED **1/11/16**

EQUIPMENT **HOLLOW STEM AUGER** BY: **RDG**

DEPTH IN FEET	SAMPLE NO.	LITHOLOGY	GROUNDWATER	SOIL CLASS (USCS)	MATERIAL DESCRIPTION	PENETRATION RESISTANCE (BLOWS/FT*)	DRY DENSITY (P.C.F.)	MOISTURE CONTENT (%)
0	BULK 0-8'				**ARTIFICIAL FILL** Sandy Clay, soft, wet, brown to dark brown, fine- to coarse-grained, trace fine gravel, trace coarse gravel.			
6				SM	**ALLUVIUM** Silty Sand, medium dense, slightly moist, yellowish brown, fine- to coarse-grained, trace fine gravel, trace clay.			
8	B2@9'			CL	Sandy Clay, stiff, slightly moist, dark brown, fine- to coarse-grained, trace fine gravel, trace clay.	29	104.8	19.0
14	B2@15'				- brown, decrease in sand content	39	106.3	17.5
18	B2@18'				BEDROCK (TOPANGA FORMATION) Siltstone and Claystone, soft, dark brown and yellowish brown, fine-grained, thinly bedded, moderately weathered, moderately fractured.	36	92.5	29.8
24	B2@25'					50 (5")	101.2	24.0
					Total depth of boring: 25½ feet Fill to 5 feet. No groundwater encountered. Backfilled with soil cuttings and tamped. *Penetration resistance for 140-pound hammer falling 30 inches by auto hammer.			

A8487-06-04 BORING LOGS.GPJ

Figure A2,
Log of Boring 6, Page 1 of 1

SAMPLE SYMBOLS

☐ ... SAMPLING UNSUCCESSFUL ◧ ... STANDARD PENETRATION TEST ■ ... DRIVE SAMPLE (UNDISTURBED)

⊠ ... DISTURBED OR BAG SAMPLE ◪ ... CHUNK SAMPLE ▼ ... WATER TABLE OR SEEPAGE

NOTE: THE LOG OF SUBSURFACE CONDITIONS SHOWN HEREON APPLIES ONLY AT THE SPECIFIC BORING OR TRENCH LOCATION AND AT THE DATE INDICATED. IT IS NOT WARRANTED TO BE REPRESENTATIVE OF SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND TIMES.

GEOCON

PROJECT NO. A8487-06-04

BORING 7

ELEV. (MSL.) **867** DATE COMPLETED **1/11/16**

EQUIPMENT **HOLLOW STEM AUGER** BY: **RDG**

DEPTH IN FEET	SAMPLE NO.	LITHOLOGY	GROUNDWATER	SOIL CLASS (USCS)	MATERIAL DESCRIPTION	PENETRATION RESISTANCE (BLOWS/FT*)	DRY DENSITY (P.C.F.)	MOISTURE CONTENT (%)
0	BULK 0-4'				**ARTIFICIAL FILL** Sandy Clay, soft, wet, brown, fine- to coarse-grained, trace fine gravel.			
2								
4				CL	**ALLUVIUM** Sandy Clay, hard, slightly moist, brown, fine- to medium-grained.			
6	B3@6'				Clayey Sand, medium dense, slightly moist, brown, fine- to coarse-grained.	50	109.6	14.2
8								
10	B3@10'			SC	- trace fine gravel, trace coarse gravel	50 (5")	90.7	8.4
12								
14	B3@13'				- no recovery, increase in gravel content, trace cobbles	40	--	--
16	B3@16'				**BEDROCK (TOPANGA FORMATION)** Siltstone and Claystone, soft, dark brown and yellowish brown, fine-grained, thinly bedded, moderately weathered, moderately fractured.	51	97.7	25.3
18								
20	B3@20'					60	100.3	24.2

Total depth of boring: 20½ feet
Fill to 2½ feet.
No groundwater encountered.
Backfilled with soil cuttings and tamped.

*Penetration resistance for 140-pound hammer falling 30 inches by auto hammer.

A8487-06-04 BORING LOGS.GPJ

Figure A3,
Log of Boring 7, Page 1 of 1

SAMPLE SYMBOLS

- ☐ ... SAMPLING UNSUCCESSFUL
- ◧ ... STANDARD PENETRATION TEST
- ■ ... DRIVE SAMPLE (UNDISTURBED)
- ⊠ ... DISTURBED OR BAG SAMPLE
- ◪ ... CHUNK SAMPLE
- ▼ ... WATER TABLE OR SEEPAGE

NOTE: THE LOG OF SUBSURFACE CONDITIONS SHOWN HEREON APPLIES ONLY AT THE SPECIFIC BORING OR TRENCH LOCATION AND AT THE DATE INDICATED. IT IS NOT WARRANTED TO BE REPRESENTATIVE OF SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND TIMES.

GEOCON

PROJECT NO. A8487-06-04

BORING 8

ELEV. (MSL.) **871** DATE COMPLETED **1/11/16**

EQUIPMENT **HOLLOW STEM AUGER** BY: **RDG**

DEPTH IN FEET	SAMPLE NO.	LITHOLOGY	GROUNDWATER	SOIL CLASS (USCS)	MATERIAL DESCRIPTION	PENETRATION RESISTANCE (BLOWS/FT*)	DRY DENSITY (P.C.F.)	MOISTURE CONTENT (%)
0	BULK 0-5'				**ARTIFICIAL FILL** Sandy Clay, soft, wet, brown, fine- to medium-grained, trace coarse-grained, trace fine gravel. - firm, slightly moist, trace silt, some cobbles, some concrete debris			
4	B4@3'			CL	**ALLUVIUM** Sandy Clay, hard, slightly moist, brown, fine- to coarse-grained, some cobbles.	64	113.4	15.0
6	BULK 5-10'							
8	B4@7'				- increase in sand content, some fine gravel, trace coarse gravel	50 (6")	102.3	12.5
10	B4@10'			CL	Clay, hard, moist, dark brown.	46	98.7	15.9
12	BULK 10-20'							
14								
16				CL	Sandy Clay, hard, slightly moist, brown, fine- to coarse-grained.			
18								
20	B4@20'			SC	Clayey Sand, very dense, light brown to brown, fine- to coarse-grained, abundant fine gravel.	50 (2")	87.3	17.0
22								
24					**BEDROCK (TOPANGA FORMATION)** Claystone, soft, dark brown to brown, fine-grained, thinly bedded, moderately weathered, moderately fractured.			
	B4@25'						81.5	27.7

Total depth of boring: 25½ feet
Fill to 3 feet.
No groundwater encountered.
Backfilled with soil cuttings and tamped.
*Penetration resistance for 140-pound hammer falling 30 inches by auto hammer.

A8487-06-04 BORING LOGS.GPJ

Figure A4,
Log of Boring 8, Page 1 of 1

SAMPLE SYMBOLS

- ☐ ... SAMPLING UNSUCCESSFUL
- ◧ ... STANDARD PENETRATION TEST
- ■ ... DRIVE SAMPLE (UNDISTURBED)
- ⊠ ... DISTURBED OR BAG SAMPLE
- ◪ ... CHUNK SAMPLE
- ▼ ... WATER TABLE OR SEEPAGE

NOTE: THE LOG OF SUBSURFACE CONDITIONS SHOWN HEREON APPLIES ONLY AT THE SPECIFIC BORING OR TRENCH LOCATION AND AT THE DATE INDICATED. IT IS NOT WARRANTED TO BE REPRESENTATIVE OF SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND TIMES.

GEOCON

PROJECT NO. A8487-06-04

BORING 9

ELEV. (MSL.) **857** DATE COMPLETED **1/11/16**

EQUIPMENT **HOLLOW STEM AUGER** BY: **RDG**

DEPTH IN FEET	SAMPLE NO.	LITHOLOGY	GROUNDWATER	SOIL CLASS (USCS)	MATERIAL DESCRIPTION	PENETRATION RESISTANCE (BLOWS/FT*)	DRY DENSITY (P.C.F.)	MOISTURE CONTENT (%)
0					**ARTIFICIAL FILL** Sandy Clay, soft, wet, brown, fine- to coarse-grained, trace fine gravel, trace coarse gravel.			
2								
4								
6	B5@5'			CL	**ALLUVIUM** Sandy Clay, stiff, slightly moist, light brown, fine- to medium-grained, trace silt.	39	74.8	19.3
8				SM	Silty Sand, very dense, slightly moist, light brown, fine- to coarse-grained, trace clay, abundant fine gravel.			

Total depth of boring: 9 feet
Boring terminated due to refusal.
Fill to 5 feet.
No groundwater encountered.
Backfilled with soil cuttings and tamped.

*Penetration resistance for 140-pound hammer falling 30 inches by auto hammer.

A8487-06-04 BORING LOGS.GPJ

Figure A5,
Log of Boring 9, Page 1 of 1

SAMPLE SYMBOLS

- ☐ ... SAMPLING UNSUCCESSFUL
- ◧ ... STANDARD PENETRATION TEST
- ■ ... DRIVE SAMPLE (UNDISTURBED)
- ⊠ ... DISTURBED OR BAG SAMPLE
- ◪ ... CHUNK SAMPLE
- ▼ ... WATER TABLE OR SEEPAGE

NOTE: THE LOG OF SUBSURFACE CONDITIONS SHOWN HEREON APPLIES ONLY AT THE SPECIFIC BORING OR TRENCH LOCATION AND AT THE DATE INDICATED. IT IS NOT WARRANTED TO BE REPRESENTATIVE OF SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND TIMES.

GEOCON

PROJECT NO. A8487-06-04

BORING 10

ELEV. (MSL.) **863** DATE COMPLETED **1/11/16**

EQUIPMENT **HOLLOW STEM AUGER** BY: **RDG**

DEPTH IN FEET	SAMPLE NO.	LITHOLOGY	GROUNDWATER	SOIL CLASS (USCS)	MATERIAL DESCRIPTION	PENETRATION RESISTANCE (BLOWS/FT*)	DRY DENSITY (P.C.F.)	MOISTURE CONTENT (%)
0	BULK 0-5'				**ARTIFICIAL FILL** Sandy Clay, soft, wet, brown, slightly moist, fine- to coarse-grained, trace fine gravel.			
2								
4					**ALLUVIUM** Silty Sand, dense, slightly moist, yellowish brown, fine- to coarse-grained.			
6	B6@5'			SM		68	89.4	17.7
8								
10	B6@10'			CL	Sandy Clay, hard, slightly moist, brown to dark brown, fine- to coarse-grained.	50 (4")	115.9	12.8

Total depth of boring: 10½ feet
Fill to 4 feet.
No groundwater encountered.
Backfilled with soil cuttings and tamped.

*Penetration resistance for 140-pound hammer falling 30 inches by auto hammer.

A8487-06-04 BORING LOGS.GPJ

Figure A6,
Log of Boring 10, Page 1 of 1

SAMPLE SYMBOLS

- ☐ ... SAMPLING UNSUCCESSFUL
- ◧ ... STANDARD PENETRATION TEST
- ■ ... DRIVE SAMPLE (UNDISTURBED)
- ⊠ ... DISTURBED OR BAG SAMPLE
- ◪ ... CHUNK SAMPLE
- ▼ ... WATER TABLE OR SEEPAGE

NOTE: THE LOG OF SUBSURFACE CONDITIONS SHOWN HEREON APPLIES ONLY AT THE SPECIFIC BORING OR TRENCH LOCATION AND AT THE DATE INDICATED. IT IS NOT WARRANTED TO BE REPRESENTATIVE OF SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND TIMES.

GEOCON

PROJECT NO. A8487-06-04

BORING 11

ELEV. (MSL.) **855** DATE COMPLETED **1/11/16**

EQUIPMENT **HOLLOW STEM AUGER** BY: **RDG**

DEPTH IN FEET	SAMPLE NO.	LITHOLOGY	GROUNDWATER	SOIL CLASS (USCS)	MATERIAL DESCRIPTION	PENETRATION RESISTANCE (BLOWS/FT*)	DRY DENSITY (P.C.F.)	MOISTURE CONTENT (%)
0					**ARTIFICIAL FILL** Sandy Clay, soft, wet, brown, fine- to coarse-grained.			
2	B7@2'			CL	**ALLUVIUM** Sandy Clay, stiff, slightly moist, brown, fine- to coarse-grained.	30	129.7	11.7
4								
6								
8	B7@7'				- hard, increase in sand content	54	79.2	6.6
10				SM	Silty Sand, medium dense, slightly moist, brown, fine- to coarse-grained, trace fine gravel.			
12	B7@11'				- no recovery, slightly moist, brown, fine- to coarse-grained, trace fine gravel	39	--	--
14								
16					**BEDROCK (TOPANGA FORMATION)** Siltstone and Claystone, soft, dark brown and yellowish brown, fine-grained, thinly bedded, moderately weathered, moderately fractured.			
	B7@17'					67	90.6	33.3

Total depth of boring: 17½ feet
Fill to 2 feet.
No groundwater encountered.
Backfilled with soil cuttings and tamped.

*Penetration resistance for 140-pound hammer falling 30 inches by auto hammer.

A8487-06-04 BORING LOGS.GPJ

Figure A7,
Log of Boring 11, Page 1 of 1

SAMPLE SYMBOLS

- ☐ ... SAMPLING UNSUCCESSFUL
- ◧ ... STANDARD PENETRATION TEST
- ■ ... DRIVE SAMPLE (UNDISTURBED)
- ⊠ ... DISTURBED OR BAG SAMPLE
- ◪ ... CHUNK SAMPLE
- ▼ ... WATER TABLE OR SEEPAGE

NOTE: THE LOG OF SUBSURFACE CONDITIONS SHOWN HEREON APPLIES ONLY AT THE SPECIFIC BORING OR TRENCH LOCATION AND AT THE DATE INDICATED. IT IS NOT WARRANTED TO BE REPRESENTATIVE OF SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND TIMES.

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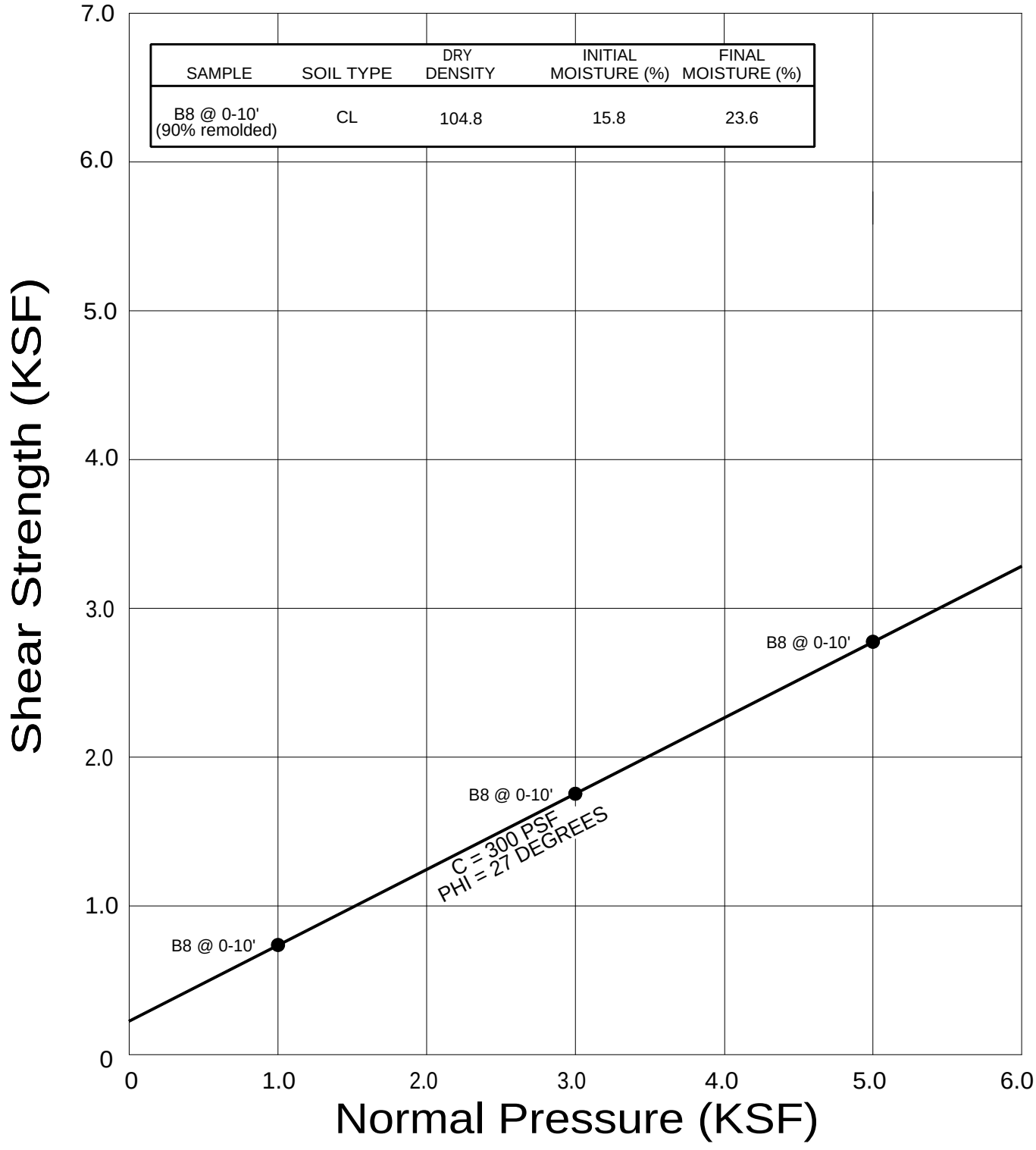
SAMPLE	SOIL TYPE	DRY DENSITY	INITIAL MOISTURE (%)	FINAL MOISTURE (%)
B8 @ 0-10' (90% remolded)	CL	104.8	15.8	23.6



● Direct Shear, Saturated

GEOCON WEST, INC.

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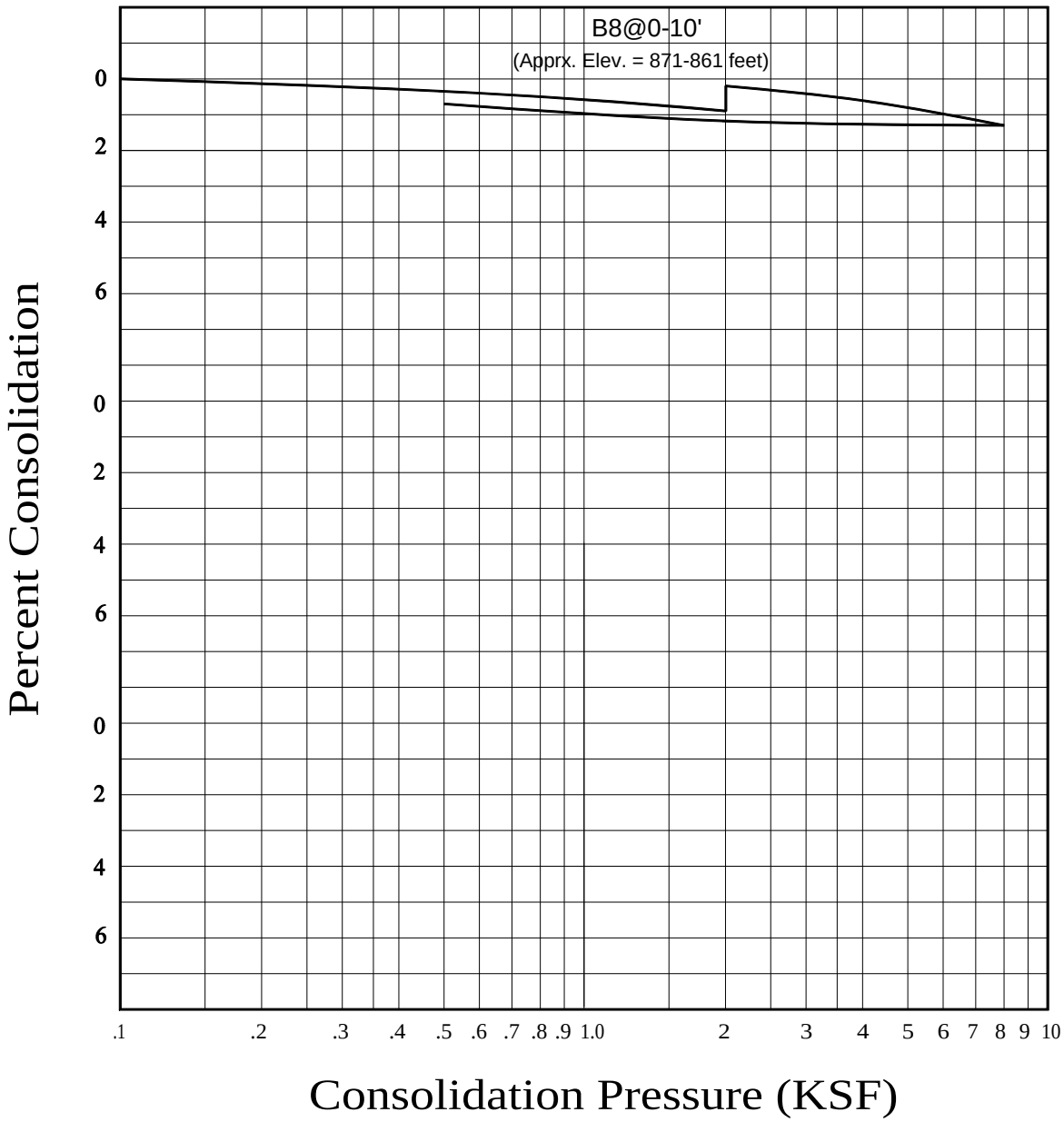
DIRECT SHEAR TEST RESULTS

AGOURA HILLS HHG HOTEL DEVELOPMENT, LP.
29508 ROADSIDE DRIVE
AGOURA HILLS, CALIFORNIA

FEB. 2016 | PROJECT NO. A8487-06-04 | FIG. B1

WATER ADDED AT 2 KSF



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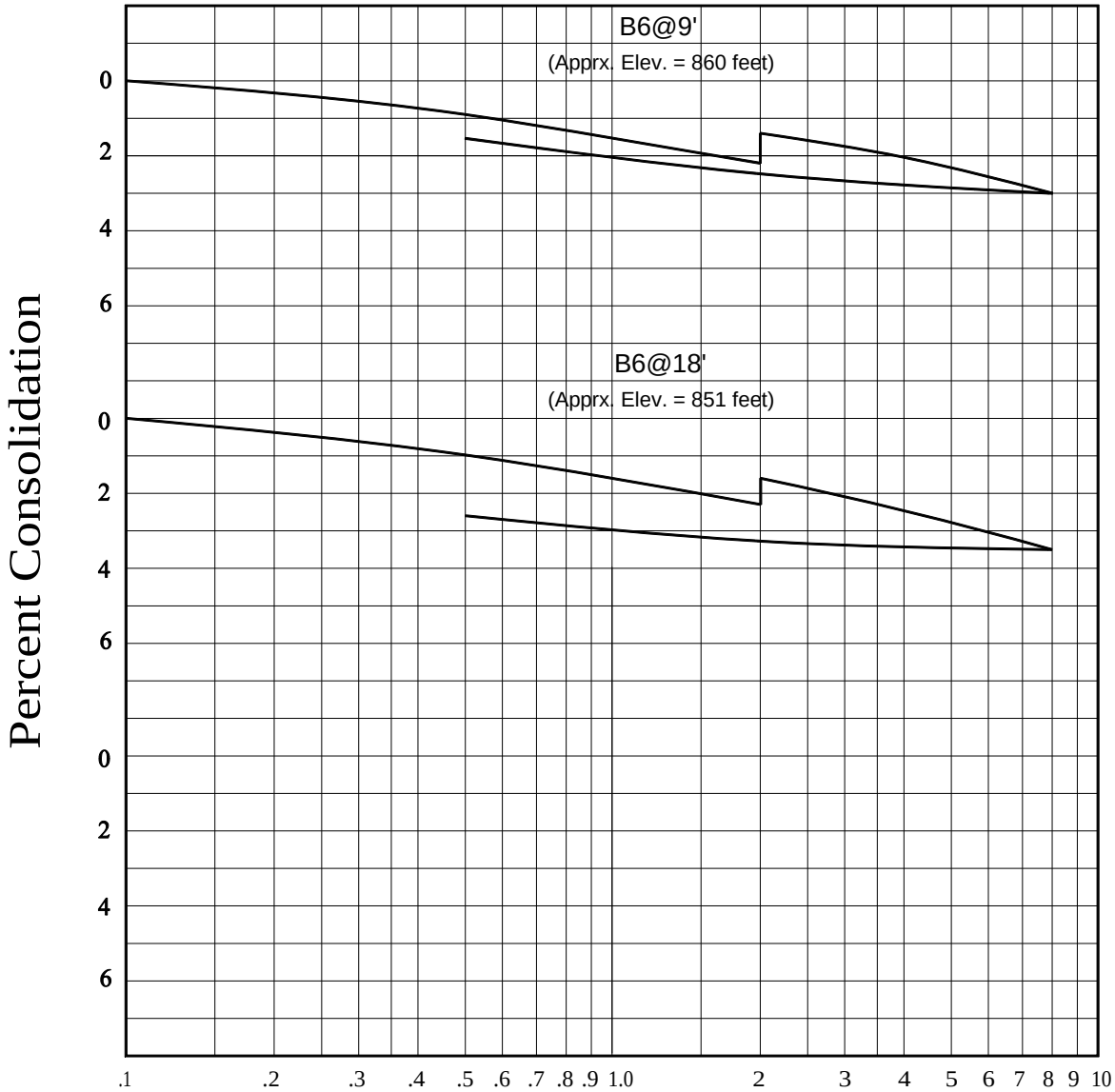
CONSOLIDATION TEST RESULTS

AGOURA HILLS HHG HOTEL DEVELOPMENT, LP.
29508 ROADSIDE DRIVE
AGOURA HILLS, CALIFORNIA

FEB. 2016	PROJECT NO. A8487-06-04	FIG. B2

WATER ADDED AT 2 KSF



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CONSOLIDATION TEST RESULTS

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FEB. 2016	PROJECT NO. A8487-06-04	FIG. B3